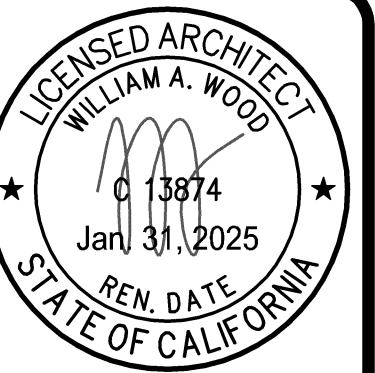


DANVILLE ACCESSORY
DWELLING UNITS
(ONE BEDROOM)
(MEDITERRANEAN STYLE)

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**DANVILLE ACCESSORY
DWELLING UNITS - ONE BEDROOM**

DANVILLE CA 94526

WILLIAM WOOD
ARCHITECTS
301 HARTZ AVENUE, SUITE 203

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GENERAL NOTES

2022 CALIFORNIA BUILDING CODE
2022 CALIFORNIA RESIDENTIAL CODE
2022 CALIFORNIA GREEN BUILDING STANDARDS CODE
2022 CALIFORNIA MECHANICAL CODE
2022 CALIFORNIA ELECTRICAL CODE
2022 CALIFORNIA PLUMBING CODE
2022 CALIFORNIA FIRE CODE
2022 CALIFORNIA ENERGY CODE
TOWN OF DANVILLE MUNICIPAL CODE

1. All materials, workmanship and methods of construction shall conform to the requirements of the 2022 codes listed above. The Contractor is required to incorporate whatever changes or additional work these codes or regulations require without additional cost to the Owner.
2. Do not scale drawings. Written dimensions shall always take precedence over scale dimensions. Dimensions shown on floor plans are to face of stud unless otherwise noted or indicated.
3. Notify the Architect of any discrepancies in the Documents or between the Documents and the existing conditions before proceeding with any work.
4. The Contractor shall obtain all required inspections for his work and give the Owner timely notice of his intent to have inspections.
5. The Contractor and/or Subcontractor shall at all times keep the premises free of accumulations of waste materials or rubbish caused by his employees or work. At the completion of the work, he shall remove all his rubbish, all of his tools, scaffolding, and surplus materials from and about the building and shall leave his work "broom clean" or its equivalent.
6. Contractor to coordinate and verify all sizes of kitchen appliances, to be selected by the Owner, and the style of the design, materials, hardware and finish of the cabinets with the Owner and cabinet-maker prior to manufacturing.
7. The escape opening has a minimum net clear opening of 5.7 square feet; minimum net clear opening height of 24 inches; and minimum net clear opening width of 20 inches. Maximum sill height to be 44" where windows are provided as means of escape or rescue.
8. All ICC evaluation reports and manufacturer specifications for fireplaces and skylights to be provided by Contractor and approved by the TOWN OF DANVILLE prior to plan installation.
9. Contractor to verify size and clearances of selected water heater and furnace for adequacy of space shown on the drawings prior to construction. Notify Architect immediately of any conflicts with space requirements for this equipment.
11. All shower and tub/shower enclosures to be provided with smooth, hard, nonabsorbent finish backing (e.g., tile over W.R. Board to 70" above drain).
12. Handrails & Guardrails: Stairways to have handrails on open side of stairs, not less than 34" nor more than 38" above the nosing treads. They should be continuous the full length of the stairs and need not extend 12" beyond top and 12" plus tread width beyond bottom tread. Ends shall be returned or shall terminate in newel posts or safety terminals. Handgrip shall not be less than $1\frac{1}{4}$ " or more than 2" in cross-sectional dimension w/ no sharp corners, height of 34 inches to 38 inches above nosing, extend continuously from top to bottom risers, and terminate at newel posts or return to walls; handgrip shall have a space not less than $1\frac{1}{2}$ " out from the wall. Open guardrail and stair railings shall have intermediate rails or an ornamental pattern such that a sphere 4" in diameter cannot pass through. All interior and exterior handrails and guardrails to conform to 2022 CRC. Guardrails shall be designed to withstand a 200-pound load applied in any direction at any point along the top rail per 2022 CRC.
13. Maximum 3.5-inch handrail projection into the required stair width. Stringers and other projections such as trim are limited to $1 - \frac{1}{2}$ inches on each side.
14. Handrail(s) is required for stairways with four or more risers.
16. Batt insulation shall have maximum flame spread of 25 and smoke development of 450. .
17. The Contractor will read and conform to the Title 24 requirements for this project. A copy of Title 24 documents can be obtained from the Owner.
18. Glass & glazing to comply with 2022 CRC.
19. Discrepancies: In the event of conflict or discrepancy in contract documents, larger quantity and higher quality shall govern unless Architect approval is given in writing.

Site & Grading Notes

1. The site plan drawing is not a property survey. All information shown in regard to boundaries of property was taken from the County Assessor's maps. The Architect does not accept responsibility for the accuracy of the data presented and drawn. The Contractor is to hire and pay for a licensed surveyor to locate the property line adjacent to the addition prior to the start of construction.
2. Non-removable backflow prevention to be provided at meter and provide non-removable vacuum breaker on all hose bibbs.
3. All utilities to be field verified by contractor. William Wood Architects assume no responsibility for accuracy of utility locations, as shown on plans.
4. Restore all existing building and site improvements, including sidewalks, curb and gutters, walks, patios, fences, landscaping, sprinklers, plumbing, etc., which have been altered or damaged by reason of the contractor's operations to a new condition, to the complete satisfaction of the owner.
5. Work to comply with TOWN OF DANVILLE ordinance for setbacks and grading requirements as shown on the site plan.
6. All downspouts, site area drains and catch basins as shown on site plan shall be collected with PVC tight line rigid wall, independent of all other drainage systems.
7. All retaining walls shall have drainage with 4" perforated PVC rigid wall and collected with PVC Tight line rigid wall.
8. A positive gradient shall be provided away from the foundation in order to provide rapid removal of the surface water runoff away from the foundation to an adequate discharge point. The grade shall fall a minimum of 6" within the first 10' measured perpendicular to the face of the foundation wall per CRC R401.3.

General Construction Notes:

1. Gypsum drywall systems with fire-resistance ratings where indicated or required are to comply with governing regulations. Provide materials and installations identical with applicable assemblies that have been tested and listed by recognized authorities, including UL. Comply with FM approval Guide where applicable.

Foundation Notes:

See structural drawing for foundation notes.

Framing Notes:

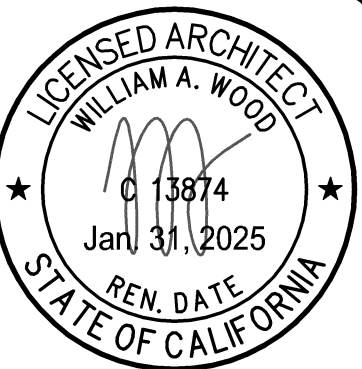
See Structural Drawings for framing notes:

Plumbing Notes

1. Showerheads shall be flow control of 1.8 G.P.M. or less.
2. All lavatory faucets shall be flow control of 1.2 G.P.M. or less.
3. Kitchen faucet to be 1.8 gallons per minute, maximum.
4. Water closet to be 1.28 gallon per flush maximum. or dual flush per CPC 411.2.
5. All piping and equipment shall be supported as per section 313 in the CPC.
6. Seismic restraint requirements for all piping and equipment (including gas lines and fuel fired equipment) shall be installed per CBC 1613.1 and Chapter 13 of ASCE7-16.
6. All backflow prevention devices shall be installed per CPC 603.3.
7. All potable and non-potable (closed loop fire sprinkler system) water piping is to be clearly identified as per the requirements of CPC 601.2.
8. Hose bibs shall be protected with a nonremovable hose-bib0-type backflow preventer, a non removable hose-bib-type vacuum breaker or by an atmospheric vacuum breaker installed not less than 6 inches above the highest point of usage located on the discharge side of the last valve. CPC 603.5.7.

Electrical and Mechanical Notes:

3. Electrical receptacles in bathrooms, outdoors and within 6'-0" of sink to be on ground fault protection (G.F.I.);
2. Provide 1/2-switched outlet under counter at sink for disposal.
4. Switch and duplex outlet, or multiple switches shown adjacent to each other on plan are to be under one plate.
5. New 110v smoke detector(s), with battery backup, and interconnected, which are audible in all sleeping area will be provided at the following locations:
(1) all new bedrooms; (2) in hallways and immediately adjacent to new bedrooms.
6. All appliances shall conform to the standards set forth by the California Energy Commission Title 24 requirements.
7. Gas fired cooking appliances and space-heating equipment shall have intermittent ignition devices.
8. All air ducts penetrating separation wall or ceiling between garage and living area shall be 26 GA. The ducts, plenums, A/C coil boxes must be insulated to R4 minimum and be of 26 ga. sheetmetal in the garage.
9. Contractor to verify space requirements for plenums and ducts prior to start of work.
10. Mechanical ventilation shall be capable of providing five air changes per hour in lieu of the natural ventilation in bathrooms, water closet compartments and laundry rooms and vented directly to the outside. All exhaust fans and fan systems to have damper controls.
11. Gaskets to be provided at exterior receptacles and switch plates.
12. Final locations of all mechanical and electrical equipment, panel boards, meters, fixtures, flues, vents, switches etc., shall be approved by Owner prior to installation.
13. Lights: All lights shall be high efficiency.
14. HVAC equipment, water heaters, showerheads and faucets to be certified by the CEC.
15. Provide a setback thermostat on all applicable heating systems.
16. Ducts penetrating the one-hour separation between garage and the house shall be not less than 26 gauge galvanized steel.
17. The HVAC equipment shown meets the assumed efficiency requirements of this compliance, however, it is recommended that the actual system to be installed be determined by an HVAC designer. The loads shown are only one of the criteria affecting the selection of HVAC equipment. Other relevant design factors such as airflow requirements, outdoor design temperatures, coil sizing, availability of equipment, oversizing safety margin, etc., must also be considered. It is the HVAC designer's responsibility to consider all factors when selecting the HVAC equipment.
18. Fused disconnect at A/C compressor unit or approved circuit breaker to be installed.
19. Clothes dryer, vented range hoods, and bathroom vent fans must be exhausted to outside per 2022 CMC.
20. Install new receptacle outlets as specified in NEC.
21. Provide a dedicated 20-amp branch circuit to supply the laundry receptacle outlet.
22. Contractor to provide additional electrical load as necessary to equip all new & existing elect. equipment.

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Professional Engineer Seal for William A. Wood, State of California, License No. C 13874, expires Jan 31, 2025.

DANVILLE CA 94526

10000 AVENUE, S
LE, CALIFORNIA
(925) 820-8233

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OF SHEETS

GENERAL NOTES

1. ALL MATERIAL AND WORKMANSHIP SHALL CONFORM TO THE 2022 EDITION OF THE CALIFORNIA BUILDING CODE (CBC); THE MOST RECENT VERSIONS OF THE CMC, CPC AND CEC; ALL APPLICABLE LOCAL CODES AND ORDINANCES; AND LOCALLY ACCEPTED STANDARDS OF PRACTICE.
2. THESE DRAWINGS AND SPECIFICATIONS HAVE BEEN PREPARED EXCLUSIVELY FOR USE ON THIS PROJECT ONLY. THE DRAWINGS AND SPECIFICATIONS, OR PORTIONS THEREOF, SHALL NOT BE USED ON OTHER PROJECTS OR ADDITIONS TO THIS PROJECT EXCEPT BY AGREEMENT IN WRITING AND WITH APPROPRIATE COMPENSATION OF THE ENGINEER.
3. WRITTEN INFORMATION AND DIMENSIONS SHALL TAKE PRECEDENCE OVER GRAPHIC INFORMATION.
4. STRUCTURAL DRAWINGS SHOW ONLY THE BASIC STRUCTURAL SYSTEMS. REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND CIVIL DRAWINGS FOR ITEMS WHICH REQUIRE SPECIAL PROVISIONS DURING CONSTRUCTION.
5. SEE DRAWINGS OTHER THAN STRUCTURAL FOR: TYPES OF FINISH MATERIALS AND THEIR LOCATIONS, FOR DEPRESSIONS IN FLOOR SLABS, FOR OPENINGS IN WALLS AND FLOORS REQUIRED BY ARCHITECTURAL AND MECHANICAL FEATURES, FOR STAIRS, CURBS, ETC.
6. ALL DETAIL REFERENCES SHALL BE CONSIDERED "TYPICAL". THE INTENT OF TYPICAL DETAILS SHALL BE APPLIED TO SIMILAR CONDITIONS ELSEWHERE IN THE PROJECT. WHEN DETAILS LABELED "SIMILAR" ARE GIVEN ON DRAWINGS, THE CONTRACTOR SHALL APPLY THE GENERAL INTENT OF THE DETAIL TO THE REFERENCED CONDITION.
7. **THE CONTRACTOR SHALL REVIEW ALL DRAWINGS IMMEDIATELY UPON RECEIPT AND SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS BEFORE STARTING WORK. THE ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCIES.**
8. STRUCTURAL DRAWINGS AND SPECIFICATIONS FOR THIS WORK HAVE BEEN PREPARED IN ACCORDANCE WITH GENERALLY ACCEPTED ENGINEERING STANDARDS OF PRACTICE TO MEET THE MINIMUM REQUIREMENTS OF THE APPLICABLE EDITION OF THE CBC. ANY OMISSIONS OR DISCREPANCIES ON THE PLANS OR ANY DEVIATIONS FROM THE PLANS THAT ARE NECESSITATED BY FIELD CONDITIONS OR ANY CONFLICTS THEREFROM SHALL BE INDICATED ON THE PLANS SHOULD BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO CONTINUING CONSTRUCTION. ALL WORK SHALL BE COORDINATED SO COOPERATION BETWEEN THE TRADES IS ACCOMPLISHED.
9. CONNECTIONS AND IMPLIED CONSTRUCTION ASSEMBLIES THAT ARE NOT SPECIFICALLY DESCRIBED OR DETAILED SHALL BE CONSTRUCTED USING STANDARD CONSTRUCTION PRACTICES IN COMPLIANCE WITH THE GOVERNING CODES AND LOCAL ORDINANCES.
10. THE STRUCTURAL SYSTEMS HAVE BEEN DESIGNED TO CARRY THE SUPERIMPOSED LIVE LOADS AS PRESCRIBED BY THE CALIFORNIA BUILDING CODE AND IN ACCORDANCE WITH STANDARD ENGINEERING PRACTICES, WITH NO SPECIAL PROVISIONS TO CARRY CONCENTRATED LOADS FROM STORAGE AND HANDLING OF CONSTRUCTION MATERIALS OR FROM OPERATION OF CONSTRUCTION EQUIPMENT.
11. DRAWINGS AND SPECIFICATIONS REPRESENT FINISHED STRUCTURE. CONTRACTOR SHALL BE RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION, INCLUDING BUT NOT LIMITED TO SHORING AND TEMPORARY BRACING. THE CONTRACTOR SHALL TAKE ALL NECESSARY MEASURE TO INSURE SAFETY OF ALL PERSONS AND STRUCTURES AT THE SITE AND ADJACENT TO THE SITE. OBSERVATION VISITS TO THE SITE BY THE ARCHITECT, ENGINEER OR CONSTRUCTION MANAGER SHALL NOT RELIEVE THE CONTRACTOR OF SUCH RESPONSIBILITY.
12. THE CONTRACTOR IS RESPONSIBLE FOR AND SHALL MAINTAIN THE INTEGRITY OF ALL SCAFFOLDING, BRACING, AND SHORING SYSTEMS AS REQUIRED FOR INSTALLATION, STABILITY AND SAFETY OF NEW WORK AND EXISTING STRUCTURES, PIPING, AND FOUNDATION SYSTEMS. CONTRACTOR SHALL ALSO PROVIDE FOR THE SAFETY OF PEDESTRIANS AND JOB SITE PERSONNEL. AT ALL TIMES, THE CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR THE CONDITION OF THE JOB SITE, INCLUDING SAFETY OF PERSONS AND PROPERTY. THE CONTRACTOR SHALL PROTECT NEW AND EXISTING CONSTRUCTION FROM INCMENET WEATHER AND PHYSICAL DAMAGE.
13. CONTRACTOR SHALL COORDINATE WITH THE CITY TO ENSURE ALL INSPECTIONS (INCLUDING SPECIAL INSPECTIONS) ARE COMPLETED PER THE LOCAL BUILDING DEPARTMENT REQUIREMENTS. APPROVALS BY BUILDING INSPECTORS SHALL NOT CONSTITUTE AUTHORITY TO DEVIATE FROM THE PLANS AND SPECIFICATIONS.
14. IF PROVIDED, OBSERVATION OF THE CONSTRUCTION BY THE ENGINEER IS INTENDED TO IMPROVE THE PROBABILITY THAT THE WORK IS COMPLETED IN GENERAL CONFORMANCE WITH THE ENGINEERING INTENT OF THE DESIGN. OBSERVATION OF THE CONSTRUCTION BY THE ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY FOR COMPLETING THE CONSTRUCTION IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS, GENERALLY ACCEPTED STANDARDS OF PRACTICE, AND CITY/COUNTY REQUIRED INSPECTIONS.
15. ALL FRAMING HARDWARE SHALL BE MANUFACTURED BY SIMPSON STRONGTIE, OR EQUAL. ALTERNATE FRAMING HARDWARE MANUFACTURERS SHALL NOT BE PROVIDED UNLESS SPECIFICALLY AUTHORIZED BY THE ENGINEER AND THE BUILDING OWNER. IF ALTERNATE HARDWARE SYSTEMS ARE AUTHORIZED, THE CONTRACTOR SHALL FORWARD COMPLETE SHOP DRAWINGS FOR REVIEW AND APPROVAL.

FOUNDATION NOTES

1. FOOTING DEPTH DIMENSION ARE INTO UNDISTURBED SOIL OR ENGINEERED FILL APPROVED BY THE GEOTECHNICAL ENGINEER, IF APPLICABLE. FINAL FOOTING DEPTH SHALL BE MEASURED FROM LOWEST ADJACENT GRADE OR BOTTOM OF UNAPPROVED FILL.
2. REMOVE LOOSE SOIL AND STANDING WATER FROM FOUNDATION EXCAVATIONS PRIOR TO PLACING CONCRETE. THE GEOTECHNICAL ENGINEER (IF APPLICABLE) SHALL INSPECT AND APPROVE ALL EXCAVATIONS, SOIL COMPACTION WORK PRIOR TO PLACEMENT OF ANY REBAR OR CONCRETE, SHORING INSTALLATIONS, BACKFILL MATERIALS AND BACK FILLING PROCEDURES.
3. MINIMUM CLEARANCE OF 8" SHALL BE PROVIDED BETWEEN EARTH AND WOOD IN ALL LOCATIONS PER CBC 2304.12
4. **DRAINAGE AND SURFACE RUNOFF:**
PERIMETER GRADES SHOULD BE POSITIVELY SLOPED AT ALL TIMES TO PROVIDE FOR RAPID REMOVAL OF SURFACE WATER RUNOFF AWAY FROM THE FOUNDATION SYSTEMS AND TO PREVENT PONDING OF WATER UNDER FOUNDATIONS OR SEEPAGE TOWARD THE FOUNDATION SYSTEMS AT ANY TIME DURING OR AFTER CONSTRUCTION. PONDED WATER MAY CAUSE UNDESIRABLE SOIL SWELL OR LOSS OF STRENGTH.
A. AS A MINIMUM REQUIREMENT, FINISHED GRADES SHOULD HAVE SLOPES OF AT LEAST 5 PERCENT WITHIN 10 FEET FROM THE EXTERIOR WALLS AND AT RIGHT ANGLES TO ALLOW SURFACE WATER TO DRAIN POSITIVELY AWAY FROM THE STRUCTURE. FOR PAVED AREAS, THE SLOPE GRADIENT CAN BE REDUCED TO 2 PERCENT.
- B. ALL SURFACE WATER SHOULD BE COLLECTED AND DISCHARGED INTO APPROVED DRAINAGE FACILITIES. APPROVED DRAINAGE FACILITIES SHALL BE PACIFIED BY THE THE CIVIL ENGINEER, IF APPLICABLE.
- C. ALL ROOF STORMWATER SHOULD BE COLLECTED AND DIRECTED TO DOWNSPOUTS. STORMWATER FROM ROOF DOWNSPOUTS SHOULD NOT BE ALLOWED TO DISCHARGE DIRECTLY ONTO THE GROUND SURFACE IN CLOSE PROXIMITY TO THE FOUNDATION SYSTEM. RATHER, STORMWATER FROM ROOF DOWNSPOUTS SHOULD BE DIRECTED BY AN IMPERMEABLE SURFACE INTO THE STREET OR TO AN APPROVED DRAINAGE FACILITY. IF THIS IS NOT ACCEPTABLE, WE RECOMMEND DOWNSPOUTS DISCHARGE AT LEAST 5 FEET AWAY FROM FOUNDATIONS.
5. **OVER-OPTIMUM SOIL MOISTURE CONDITIONS:**
THE CONTRACTOR SHOULD ANTICIPATE ENCOUNTERING EXCESSIVELY OVER-OPTIMUM (WET) SOIL MOISTURE CONDITIONS DURING WINTER OR SPRING GRADING, OR DURING OR FOLLOWING PERIODS OF RAIN. IN ADDITION, WET SOIL CONDITIONS MAY BE ENCOUNTERED NEAR THE BOTTOM OF EXCAVATIONS. WET SOIL CAN MAKE PROPER COMPACTION DIFFICULT OR IMPOSSIBLE. WET SOIL CONDITIONS SHALL BE MITIGATED BY APPROVED MEANS.
6. **OVER-OPTIMUM SOIL MOISTURE CONDITIONS:**
WHERE MOISTURE VAPOR TRANSMISSION IS A CONCERN, CONSULT A WATERPROOFING EXPERT. MOISTURE TRANSITION IS OUTSIDE THE SCOPE OF THESE PLANS.
A. A TIGHT, WATER VAPOR RETARDING MEMBRANE SHOULD BE INSTALLED BELOW ALL SLAB FOUNDATIONS SYSTEMS TO REDUCE MOISTURE CONDENSATION UNDER FLOOR COVERINGS. THE VAPOR RETARDER SHOULD MEET ASTM E 1745 CLASS A REQUIREMENTS FOR WATER VAPOR PERMEANCE, TENSILE STRENGTH, AND PUNCTURE RESISTANCE. VAPOR TRANSMISSION THROUGH THE SLAB FOUNDATIONS CAN ALSO BE REDUCED BY USING HIGH STRENGTH CONCRETE WITH A LOW WATER-CEMENT RATIO.
7. DEEPEN PERIMETER FOOTINGS AS REQUIRED WHERE FOOTING, OR EDGE OF SLAB, IS WITHIN 3'-0" OF EDGE OF BIO-SWALE, BIO-RETENTION FACILITIES, TRENCHES, ETC. DEEPEN FOOTING SUCH THAT A 1:1 PLANE IS MAINTAINED BETWEEN BOTTOM OF FOOTING AND BOTTOM OF ADJACENT EXCAVATION. SEE DTL. 207 FOR MORE INFORMATION.
8. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE AND PROTECT EXISTING UTILITIES TO REMAIN DURING AND/OR AFTER CONSTRUCTION.
9. CONTRACTOR TO REMOVE ABANDONED FOOTINGS, UTILITIES, ETC. WHICH INTERFERE WITH NEW CONSTRUCTION, UNLESS OTHERWISE INDICATED. NOTIFY THE OWNER'S REPRESENTATIVE IF ANY BURIED STRUCTURES NOT INDICATED, SUCH AS CESSPOOLS, CISTERNS, FOUNDATIONS, ETC., ARE FOUND.
10. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, UNDERPINNING AND PROTECTION OF EXISTING CONSTRUCTION.
11. PLACE BACKFILL BEHIND RETAINING WALLS AFTER CONCRETE OR MASONRY HAS ATTAINED FULL DESIGN STRENGTH. BRACE BUILDING AND PIT WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHED FLOORS AND SLABS ON GRADE ARE COMPLETE AND HAVE ATTAINED FULL DESIGN STRENGTH.

SYMBOLS LEGEND

- NOT ALL SYMBOLS USED IN THE DRAWING ARE LISTED BELOW. REFER TO AMERICAN WELDING SOCIETY PUBLICATIONS FOR WELDING SYMBOLS.
- (LENGTH)
SW-# INDICATES SHEARWALL NUMBER AND MINIMUM DESIGN LENGTH. SHEARWALLS SHALL BE AS NOTED IN THE SHEARWALL SCHEDULE
- SD INDICATES HOLDOWN TYPE AS MANUFACTURED BY THE "SIMPSON STRONGTIE COMPANY." HOLDOWNS SHALL BE AS NOTED IN THE HOLDOWN SCHEDULE, PER
- INDICATES INTERIOR BEARING WALL BELOW

SHEARWALL NOTES

1. WHERE A STRUCTURAL SHEARWALL IS INDICATED ON PLANS THE ASSEMBLY SHALL RUN HORIZONTALLY AND CONTINUOUSLY TO THE NEAREST WALL OPENING OR END OF THE WALL; THE ASSEMBLY SHALL RUN VERTICALLY CONTINUOUSLY FROM THE BOTTOM OF THE NEAREST SOLE OR BOTTOM PLATE UP TO THE TOP OF THE NEAREST DOUBLE TOP PLATE (OR BEAM). ALL PLYWOOD PANEL EDGES SHALL BE BLOCKED AND EDGE NAILED.
2. WHERE HOLDOWN POSTS OR STUDS ARE INDICATED AT THE END OF A SHEARWALL, THE SHEAR PLYWOOD SHALL BE EDGE NAILED AND THE POST SHALL RUN CONTINUOUSLY FROM THE SOLE PLATE TO THE DOUBLE TOP PLATE. HOLDOWNS SHALL BE ATTACHED TO POSTS AT THE ENDS OF SHEARWALLS AND SHALL EXTEND TO EITHER FRAMING BELOW OR TO FOUNDATION AS SHOWN ON PLANS.
3. SEE SHEARWALL SCHEDULE ON PLANS FOR REQUIRED SHEARWALL NAILING, ANCHOR BOLTS, SILL NAILS, AND OTHER SHEAR TRANSFER HARDWARE.
4. SHEARWALL PLYWOOD SHALL NOT BE CUT FOR PIPE, DUCTS, SLEEVES, ETC., U.N.O. OR DETAILED.
5. UNLESS OTHERWISE DETAILED, ALL INTERIOR SHEARWALLS SHALL BE CONTINUOUS TO THE ROOF OR FLOOR PLYWOOD IN ACCORDANCE WITH THE TYPICAL SHEAR TRANSFER DETAILS
6. PLYWOOD SHEETS LOCATED AT SHEARWALL EDGES SHALL BE AT LEAST 12" WIDE. PLYWOOD EDGES SHALL BE EDGE NAILED TO ALL SHEARWALL FRAMING MEMBERS. SEE SHEARWALL SCHEDULE FOR FIELD NAILING REQUIREMENTS.
7. SEE SHEARWALL SCHEDULE FOR SHEARWALLS THAT REQUIRE 3x MUDDSILLS AND 3x FRAMING AT ADJOINING PLYWOOD PANEL EDGES. SILL PLATES, TOP PLATES AND MEMBERS IN THE FIELD OF INDIVIDUAL PLYWOOD PANELS DO NOT TYPICALLY BACK ADJOINING PANEL EDGES AND THUS MAY BE 2x.

OBSERVATION OF CONSTRUCTION

1. IN ADDITION TO OBSERVATIONS BY THE SOILS ENGINEER(IF APPLICABLE), AND CITY INSPECTOR, OBSERVATION OF THE CONSTRUCTION BY THE PROJECT ENGINEER IS RECOMMENDED FOR THIS PROJECT. THE CONTRACTOR SHALL PHASE THE PROJECT AND COORDINATE WITH THE ENGINEER TO ENSURE THAT THE PRIMARY STRUCTURAL ELEMENTS OF THE CONSTRUCTION ARE OBSERVED PRIOR TO COVERING WITH FINISHES OR OTHER MATERIALS. AS A MINIMUM, OBSERVATION BY THE ENGINEER IS RECOMMENDED AS FOLLOWS:
- A. REINF. STEEL AND HARDWARE EMBEDDED IN THE FNDTION SHALL BE OBSERVED PRIOR TO CONCRETE PLACEMENT
B. SHEARWALLS AND FRAMING ELEMENTS SHALL BE OBSERVED PRIOR TO INSTALLATION OF FINISHES.
2. OBSERVATION OF THE CONSTRUCTION BY THE ENGINEER DOES NOT RELIEVE THE CONTRACTOR FROM RESPONSIBILITY TO COMPLETE THE CONSTRUCTION IN CONFORMANCE WITH THE PROJECT DOCUMENTS AND GENERALLY ACCEPTED STANDARDS OF PRACTICE. THE PURPOSE OF OUR VISITS WILL BE TO BECOME GENERALLY FAMILIAR WITH THE PROGRESS AND QUALITY OF THE CONTRACTOR'S WORK AND DETERMINE IF THE WORK IS PROGRESSING IN GENERAL CONFORMANCE WITH OUR DESIGN INTENT. DURING OUR VISITS, ADVANCED ENGINEERING WILL NOT BE MAKING DETAILED INSPECTIONS, OR VERIFYING DIMENSIONS. THE CONTRACTOR IS RESPONSIBLE FOR FOLLOWING APPLICABLE CODES AND THE APPROVED CONSTRUCTION DOCUMENTS.

SPECIAL INSPECTIONS PER 2022 CBC 1701

1. IN ADDITION TO OBSERVATIONS BY THE CITY INSPECTOR AND THE PROJECT ENGINEER, SPECIAL INSPECTIONS BY AN ICC CERTIFIED SPECIAL INSPECTOR IS REQUIRED AS FOLLOWS:
- A. EPOXY ANCHORS INSTALLED IN CONCRETE SHALL HAVE SPECIAL INSPECTION OF HOLE DEPTH & DIAMETER, MATERIALS, CLEANING PROCEDURES, AND INSTALLATION PER ICC-REPORT ICC-ES/ESR-2508 SECTION 4.4.3.
B. TITEN HD ANCHORS INSTALLED IN CONCRETE SHALL HAVE SPECIAL INSPECTION OF DRILLING AND INSTALLATION PER ICC-REPORT ICC-ES/ESR-2713 SECTION 4.4.
C. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR NAILING, BOLTING, ANCHORING, AND OTHER FASTENING OF COMPONENTS WITHIN THE SEISMIC-FORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR PANELS AND HOLDOWNS PER SECTION 1705.12.2 OF THE CURRENT EDITION OF THE C.B.C.
D. CONTINUOUS INSPECTIONS OF CAST-IN-PLACE DEEP FOUNDATION (PIERS) OR DRILLING OPERATIONS. COMPLETE AND ACCURATE RECORDS FOR EACH ELEMENT SHALL BE PROVIDED PER SECTION 1705.8 OF THE CURRENT EDITION OF THE C.B.C.
E. CONTINUOUS INSPECTIONS OF CAST-IN-PLACE DEEP FOUNDATION (PIERS) TO VERIFY: PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM ELEMENT DIAMETERS, LENGTHS, AND EMBEDMENTS PER SECTION 1705.8 OF THE CURRENT EDITION OF THE C.B.C.
2. THE CONTRACTOR SHALL COORDINATE WITH THE CITY TO ENSURE SPECIAL INSPECTION IS PROVIDED PER CITY REQUIREMENTS. THE SPECIAL INSPECTOR SHALL BE EMPLOYED BY THE OWNER AND MUST DEMONSTRATE HIS QUALIFICATIONS TO THE ARCHITECT/ENGINEER OF RECORD AND THE BUILDING OFFICIAL.

FASTENING SCHEDULE (C.B.C. TABLE 2304.10.2)

ITEM	DESCRIPTION OF CONNECTION	FASTENING (NOTE 1, 4, 8)		
		COMMON / BOX	SMOOTH SHANK	STAPLES
1	JOIST TO SILL OR GIRDER, TOE-NAIL	(3) 8d	(3) 3" x 0.131"	(3) 3"-14 GA.
2	BRIDGING TO JOIST, TOE-NAIL EA. END	(2) 8d	(2) 3" x 0.131"	(2) 3"-14 GA.
3	1" x 6" SUBFLOOR OR LESS, TO EACH JOIST, FACE NAIL	(2) 8d	N.A.	N.A.
4	WIDER THAN 1" x 6" SUBFLOOR TO EACH JOIST, FACE NAIL	(3) 8d	N.A.	N.A.
5	2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	(2) 16d	N.A.	N.A.
6	SOLE PLATE TO JOIST OR BLOCKING, TYPICAL FACE NAIL	16d AT 16"	3" x 0.131" AT 8" O.C.	3"-14 GA. AT 12" O.C.
	SOLE PLATE TO JOIST OR BLOCKING, AT BRACED WALL PANELS	(3) 16d AT 16"	(4) 3" x 0.131" AT 16" O.C.	(4) 3"-14 GA. PER 16" O.C.
7	TOP PLATE TO STUD, END NAIL	(2) 16d	(3) 3" x 0.131"	(3) 3"-14 GA.
8	STUD TO SOLE PLATE, END NAIL	(2) 16d	(3) 3" x 0.131"	(3) 3"-14 GA.
	STUD TO SOLE PLATE, TOE-NAIL	(4) 8d	(4) 3" x 0.131"	(3) 3"-14 GA.
9	DOUBLE STUDS, FACE NAIL	16d AT 24"	3" x 0.131" AT 8" O.C.	3"-14 GA. AT 8" O.C.
10	DOUBLED TOP PLATES, TYP. FACE NAIL	16d AT 16"	3" x 0.131" AT 12" O.C.	3"-14 GA. AT 12" O.C.
	DOUBLED TOP PLATES, LAP SPLICE	(8) 16d	(12) 3" x 0.131"	(12) 3"-14 GA.
11	BLOCKING BETWEEN JOISTS OR RAFTERS, TO TOP PLATE, TOE-NAIL	(3) 8d	(3) 3" x 0.131"	(3) 3"-14 GA.
12	RIM JOIST TO TOP PLATE, TOE-NAIL	8d AT 6"	3" x 0.131" AT 6" O.C.	3"-14 GA. AT 6" O.C.
13	TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	(2) 16d	(3) 3" x 0.131"	(3) 3"-14 GA.
14	CONTINUOUS HEADER, TWO PIECES	16d AT 16" ALONG EDGE	N.A.	N.A.
15	CEILING JOIST TO PLATE, TOE-NAIL	(3) 8d	(5) 3" x 0.131"	(5) 3"-14 GA.
16	CONTINUOUS HEADER TO STUD, TOE-NAIL	(4) 8d	N.A.	N.A.
17	CEILING JOISTS: LAPS OVER PARTITIONS, FACE NAIL (SEE C.B.C. TABLE 2308.7.3.1)	(3) 16d MIN.	(4) 3" x 0.131"	(4) 3"-14 GA.
18	CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL (SEE C.B.C. TABLE 2308.7.3.1)	(3) 16d MIN.	(4) 3" x 0.131"	(4) 3"-14 GA.
19	RAFTER TO PLATE, TOE-NAIL (SEE C.B.C. TABLE 2308.7.9)	(2) 8d	(3) 3" x 0.131"	(3) 3"-14 GA.
20	1" DIAGONAL BRACE TO EACH STUD AND PLATE, FACE NAIL	(3) 8d	(2) 3" x 0.131"	(3) 3"-14 GA.
21	1" x 8" SHEATHING OR LESS TO EACH BEARING, FACE NAIL	(3) 8d	N.A.	N.A.
22	WIDER THAN 1" x 8" SHEATHING TO EACH BEARING, FACE NAIL	(3) 8d	N.A.	N.A.
23	BUILT-UP CORNER STUDS	16d AT 24"	3" x 0.131" AT 16" O.C.	3"-14 GA. AT 16" O.C.
24	BUILT-UP GIRDER AND BEAMS, FACE NAIL TOP AND BTM STAGG. ON OPP. SIDE	20d AT 32"	3" x 0.131" AT 24" O.C.	3"-14 GA. AT 24" O.C.
	BUILT-UP GIRDER AND BEAMS, FACE NAIL AT ENDS AND AT EACH SPLICE	(2) 20d	(3) 3" x 0.131"	(3) 3"-14 GA.
25	2" PLANK TO EACH BEARING	16d	N.A.	N.A.
26	COLLAR TIE TO RAFTER, FACE NAIL	(3) 10d	(4) 3" x 0.131"	(4) 3"-14 GA.
27	JACK RAFTER TO HIP, TOE-NAIL	(3) 10d	(4) 3" x 0.131"	(4) 3"-14 GA.
	JACK RAFTER TO HIP, FACE NAIL	(2) 16d	(3) 3" x 0.131"	(3) 3"-14 GA.
28	ROOF RAFTER TO 2x RIDGE BEAM, TOE-NAIL OR FACE-NAIL	(2) 16d	(3) 3" x 0.131"	(3) 3"-14 GA.
29	JOIST TO BAND JOIST, FACE NAIL	(3) 16d	(4) 3" x 0.131"	(4) 3"-14 GA.
30	LEDGER STRIP, FACE NAIL	(3) 16d	(4) 3" x 0.131"	(4) 3"-14 GA.
31	WOOD STRUCTURAL PANELS AND PARTICLE BOARD (NOTE 2): SUBFLOOR, ROOF + WALL SHEATHING TO FRAMING:			
	1/2" AND LESS	6d (NOTE 3, 7)	2.375" x 0.113" (NOTE 9)	1.75" x 16 GA. (NOTE 10)
	19/32" TO 3/4"	8d (NOTE 4)	2.375" x 0.113" (NOTE 11)	2" x 16 GA. (NOTE 11)
	7/8" TO 1"	8d (NOTE 3)	N.A.	N.A.
	1-1/8" TO 1-1/4"	10d OR 8d (NOTE 4)	N.A.	N.A.
	SINGLE FLOOR (COMBINATION SUBFLOOR- UNDERLAYMENT TO FRAMING):			
	3/4" AND LESS	6d (NOTE 5)	N.A.	N.A.
	7/8" TO 1"	8d (NOTE 5)	N.A.	N.A.
	1-1/8" TO 1-1/4"	10d (NOTE 4)	N.A.	N.A.
	PANEL SIDING TO FRAMING (NOTE 2):			
32	1/2" AND LESS	6d (NOTE 6)	N.A.	N.A.
	5/8"	8d (NOTE 6)	N.A.	N.A.

NOTES:

- FOR S1: 1 inch = 25.4 mm
1. COMMON OR BOX NAILS ARE PERMITTED TO BE USED EXCEPT WHERE OTHERWISE STATED.
2. NAILS SPACED AT 6" O.C. AT EDGES, 12" O.C. AT INTERMEDIATE SUPPORTS EXCEPT 6" O.C. AT ALL SUPPORTS WHERE SPANS ARE 4'-0" OR MORE. NAILING OF WOOD STRUCTURAL PANEL AND PARTICLEBOARD DIAPHRAGMS AND SHEAR WALLS, REFER TO C.B.C. SECTION 2305. WALL SHEATHING NAILS ARE PERMITTED TO BE COMMON, BOX OR CASING. COMMON OR DEFORMED SHANK (6d - 2" x 0.113"; 8d - 2.5" x 0.131"; 10d - 3" x 0.148"; 16d - 3.5" x 0.162")
3. COMMON (6d - 2" x 0.113"; 8d - 2.5" x 0.131"; 10d - 3" x 0.148"; 16d - 3.5" x 0.162")
4. DEFORMED SHANK (6d - 2" x 0.113"; 8d - 2.5" x 0.131"; 10d - 3" x 0.148")
5. CORROSION-RESISTANT SIDING (6d - 1.875" x 0.106"; 8d - 2.375" x 0.128") OR CASING (6d - 2" x 0.099"; 8d - 2.5" x 0.113") NAILS.
7. FOR ROOF SHEATHING APPLICATIONS, 8d NAILS (2.5" x 0.113") ARE THE MINIMUM REQUIRED FOR WOOD STRUCTURAL PANELS.
8. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16".
9. FOR ROOF SHEATHING APPLICATIONS, FASTENERS SPACED 4" O.C. AT EDGES AND 8" O.C. AT INTERMEDIATE SUPPORTS.
10. FASTENERS SPACED 4" O.C. AT EDGES AND 8" O.C. AT INTERMEDIATE SUPPORTS FOR SUBFLOOR AND WALL SHEATHING AND 3" O.C. AT EDGES AND 6" O.C. AT INTERMEDIATE SUPPORTS FOR ROOF SHEATHING.
11. FASTENERS SPACED 4" O.C. AT EDGES AND 8" O.C. AT INTERMEDIATE SUPPORTS.

NAIL SCHEDULE

FASTENER PER PLAN	SHANK DIA.	HEAD DIA.	MIN. LENGTH
8d	0.131"	0.281"	2-1/2"
10d	0.148"	0.312"	3"
16d	0.148"	0.344"	3-1/4"
16d COMMON	0.162"	0.344"	3-1/2"
20d	0.192"	0.406"	4"

INDICATES POST BELOW

INDICATES POST ABOVE

DETAIL NUMBER

SHEET NUMBER

INDICATES ELEVATION

REFERENCE TO STRUCTURAL CALCULATIONS FOR THE BENEFIT OF THE BUILDING OFFICIAL

INDICATES REVISION NUMBER AND PLAN REVISIONS INSIDE CLOUD

REFERENCE TO STRUCTURAL CALCULATIONS FOR THE BENEFIT OF THE BUILDING OFFICIAL

INDICATES HANGER.

ABBREVIATION LEGEND

A.B.	ANCHOR BOLT	EXT.	EXTERIOR	OPNG	OPENING
ABV	ABOVE	FDN	FOUNDATION	O.S.B.	ORIENTED STRAND BOARD
ACI	AMERICAN CONCRETE	F.F.	FINISH FLOOR	O.W.S.J.	OPEN WEB STEEL JOIST
	INSTITUTE	FIN.	FINISH	O.W.J.G.	OPEN WEB JOIST GIRDER
ADD'L	ADDITIONAL	F.J.	FLOOR JOIST	P.A.	POST ABOVE
A.F.F.	ABOVE FINISHED FLOOR	FLG	FLANGE	P.A.F.	POWDER-ACTUATED
AGGREG.	AGGREGATE	FLR	FLOOR		FASTENER
AISC	AMERICAN INSTITUTE OF	F.O.B.	FACE OF BLOCK	P.C.F.	POUNDS PER CUBIC FOOT
	STEEL CONSTRUCTION	F.N.	FACE NAIL	PEN.	PENETRATION
ATC	AMERICAN INSTITUTE OF	F.O.C.	FACE OF CONCRETE	PERF.	PERFORATED
	TIMBER CONSTRUCTION	F.O.M.	FACE OF MASONRY	PERP.	PERPENDICULAR
ALT.	ALTERNATE	F.O.S.	FACE OF STUD	PL.	PLATE
ARCH.	ARCHITECT(URAL)	F.P.	FIREPLACE - OR - FULL PENETRATION	PLY.	PLYWOOD
ASTM	AMERICAN SOCIETY FOR	FRMD	FRAMED	PLYWD	PLYWOOD
	TESTING & MATERIALS	FRMG	FRAMING	PS.	PAIR
B/	BOTTOM OF	F.S.	FAR SIDE	P.S.F.	POUNDS PER SQUARE FOOT
B.C.	BOTTOM CHORD	F.T.	FOOT	P.S.I.	POUNDS PER SQUARE INCH
BLDG	BUILDING	FT	FOOT	P.S.L.	PARALLEL-STRAND LUMBER
BLK	BLOCK	FTG	FOOTING	PST	POST
BLKG	BLOCKING	G & N	GLUE AND NAIL	PT	POINT
BLW	BELOW	GA.	GAGE	P.T.	PRESSURE TREATED - OR -
BR	BEAM	GALV.	GALVANIZED	R.	RADIUS
B.N.	BOUNDARY NAILING	G.B.	GRADE BEAM	REC'D	RECOMMEND(ATION)
B.O.	BY OTHERS - OR -	GEN.	GENERAL	REF.	REFERENCE
	BOTTOM OF	GLB	GLUED-LAMINATED BEAM	REINF.	REINFORCEMENT
		GR.	GRADE		REINFORCED - OR -
B.O.C.	BOTTOM OF CONCRETE	G.T.	GIRDER TRUSS		REINFORCING - OR -
B.O.F.	BOTTOM OF FOOTING	GYP.	GYPNUM		REINFORCEMENT
B.O.S.	BOTTOM OF STEEL	G.W.B.	GYPNUM WALL BOARD	REQD	REQUIRED
BRG	BEARING	HD	HOLDOWN	RET.	RETAINING
B.S.	BOTH SIDES	H.D.G.	HOT-DIP GALVANIZED	RETAIN.	RETAINING
BTM	BOTTOM	HDR	HEADER	R.J.	ROUGH JOIST
BTWN	BETWEEN	HGR	HANGER	R.O.	ROUGH OPENING
CALCS	CALCULATIONS	H.N.H.	HEAVY HEX NUT	R.P.	REAL POST
CANT.	CANTILEVER	H.N.	HEX NUT	R.S.	ROUGH RAFTER
C.B.	CEILING BEAM	HORIZ.	HORIZONTAL	R/R	RESAWN (ROUGH-SAWN)
C.B.C.	CALIFORNIA BUILDING CODE	H.S.B.	HIGH STRENGTH BOLT	R.T.	ROUGH TRUSS
		HSS	STRUCTURAL TUBE	R.W.	RETAINING WALL
C.C.	CENTER TO CENTER	HT	HEIGHT	RWD	REDWOOD
C.C.J.	CRACK CONTROL JOINT	I.D.	INSIDE DIAMETER	S.A.D.	SEE ARCHITECTURAL
C.C.	CUBIC FOOT	INFO.	INFORMATION		DRAWINGS
C.J.	CONTROL JOINT - OR -	INT.	INTERIOR	S.B.	SET-BACK TRUSS
	CEILING JOIST	INV.	INVERT(ED)	S.B.T.	SET-BACK TRUSS
Ç	CENTERLINE	JNT	JOINT	SCHED.	SCHEDULE
CLG	CEILING	JST	JOIST	SEP.	SEPARATION
CLR	CLEAR	J.T.	JACK TRUSS	S.G.T.	SUB-GIRDER TRUSS
C.M.U.	CONCRETE MASONRY UNIT	K.	KIPS	SH	SHEET
COL.	COLUMN	K.D.	KILN-DRIED	SIM.	SIMILAR
COLL.	COLLECTOR	K.LF	KIPS PER LINEAL FOOT	S.M.S.	SHEET METAL SCREW
CONC.	CONCRETE	K.P.	KING POST	S.J.	SOFFIT JOIST
COND.	CONDITION	lbs	POUNDS	SPEC.	SPECIFICATION
CONN.	CONNECTION	L.B.	LAC BOLT	SQ.	SQUARE
CONSTR.	CONSTRUCTION	L.F.	LINEAL FOOT	S.S.	SELECT STRUCTURAL
CONT.	CONTINUOUS	LONGIT.	LONGITUDINAL	S4S	SURFACED 4 SIDES
CONTR.	CONTRACTOR	LG	LONG	STAGG.	STAGGER - OR - STAGGERED
C.P.	COMPLETE PENETRATION	L.L.	LIVE LOAD	STD	STANDARD
		LSL	LAMINATED STRAND LUMBER	STIFF.	STIFFENER
C.S.	COUNTERSUNK	L.T.	LIGHT	STL	STEEL
C.T.	COLLECTOR TRUSS	LWT	LIGHTWEIGHT	STRUCT.	STRUCTURAL
CTR	CENTER	LVL	LAMINATED VENEER LUMBER	SW	SHEARWALL
CTRD	CENTERED	MATL	MATERIAL	SYMM.	SYMMETRICAL
C.Y.	CUBIC YARD	MAX.	MAXIMUM	T + B	TOP AND BOTTOM
DBL	DOUBLE	M.B.	MACHINE BOLT	T + G	TONGUE AND GROOVE
DET.	DETAIL	MECH.	MECHANICAL	T.C.	TOP CHORD
D.F.	DOUGLAS FIR	MEMB.	MEMBRANE	THK	THICK
DIA.	DIAMETER	MID.	MIDDLE	THKND	THICKENED
DIAG.	DIAGONAL	MIN.	MINIMUM	T.N.	TOE NAIL
DIM.	DIMENSION	M.I.W.	MALLEABLE IRON WASHER	T.O.B.	TOP OF BLOCK
DLD	DEAD LOAD	M.O.	MASONRY OPENING	T.O.C.	TOP OF CONCRETE
D.J.	DECK JOIST	MOIST.	MOISTURE	T.O.P.	TOP OF PLATE
D.P.	DRAFT TRUSS	MFR	MANUFACTURER	T.O.S.	TOP OF STEEL - OR -
DT	DEEP	MRFD	MANUFACTURED		TOP OF SHEATHING
DTL	DETAIL	MTL	METAL	T.O.W.	TOP OF WALL
DWG	DRAWING	(N)	NEW	T/	TOP OF
EX.	EXISTING	N.A.	NOT APPLICABLE	T.P.	TOP PLATE
E.A.	EACH	N.B.	NON-BEARING	TRANSV.	TRANSVERSE
E.B.	EXPANSION BOLT	N.D.T.	NON-DESTRUCTIVE TESTING	TYP.	TYPICAL
E.F.	EACH FACE	N.I.C.	NOT IN CONTRACT	U.N.O.	UNLESS NOTED OTHERWISE
E.J.	EXPANSION JOINT	NLG	NAILING	VERT.	VERTICAL
ELEV.	ELEVATION	No.	NUMBER	V.I.F.	VERIFY IN FIELD
ELEV.	ELEVATOR	N.S.	NEAR SIDE	WTH	WITH
EMB.	EMBEDMENT	N.S.G.	NON-SHRINK GROUT	WD	WOOD
EMBED.	EMBEDMENT	N.T.S.	NOT TO SCALE	W.P.	WORK POINT - OR -
E.N.	EDGE NAIL	O/	OVER		WATERPROOF(ING)
E.O.R.	ENGINEER OF RECORD	O.C.	ON CENTER	WT	WEIGHT
E.Q.	EQUAL	O.D.	OUTSIDE DIAMETER	W.W.F.	WELDED WIRE FABRIC
EQUIP.	EQUIPMENT	F.O.S.	OUTSIDE FACE OF STUD	W.W.M.	WELDED WIRE MESH
E.S.	EACH SIDE	O.V.	OVERHANG - OR -	#	NUMBER (PREFIX)
E.W.	EACH WAY	O.P.	OPPOSITE HAND	#	POUNDS (SUFFIX)

GENERAL FRAMING NOTES

1. NO STRUCTURAL MEMBER SHALL BE CUT OR NOTCHED UNLESS SPECIFICALLY SHOWN, NOTED, OR APPROVED BY THE ENGINEER. NOTCH DETAILS, IF PROVIDED, ARE FOR GENERAL GUIDANCE ONLY. THE ENGINEER SHALL BE CONTACTED TO APPROVE LOCATIONS OF PROPOSED NOTCHES. STUDS IN EXTERIOR WALLS AND BEARING PARTITIONS MAY BE CUT OR NOTCHED TO A DEPTH NOT EXCEEDING 25% OF STUD WIDTH. CUTTING OR NOTCHING OF STUDS IN NON-BEARING PARTITIONS SHALL NOT EXCEED 40% OF THE WIDTH. SEE DETAIL 404 FOR MORE INFORMATION.
2. **TYPICAL FRAMING AND STRUCTURAL MATERIALS:**
ALL FRAMING LUMBER SHALL BE DOUGLAS FIR, AND SHALL BE STAMPED WITH A GRADE MARK WITH THE FOLLOWING GRADES. FRAMING LUMBER SHALL CONFORM TO GRADING RULES OF WVPA AND COMPLIES WITH DOC PS 20.
MAXIMUM MOISTURE CONTENT SHALL NOT EXCEED 19% AT THE TIME OF CONSTRUCTION.
- | FRAMING MATERIAL | GRADE | FRAMING MATERIAL | GRADE |
|------------------------------------|------------------------|--|----------------------|
| TYPICAL WALL STUDS | PER NOTE 6 BELOW | RAFTERS (2x DIMENSIONAL) | D.F. #2, MIN. U.N.O. |
| SOLE PLATES (2x MIN.) ¹ | STUD GRADE, MINIMUM | BUILT UP HEADERS (2-2x) | D.F. #2, MIN. U.N.O. |
| MUDSILLS (2x MIN.) ² | P.T. D.F. STD & BETTER | 4x POSTS/BEAMS/RAFTERS | D.F. #2, MIN. U.N.O. |
| TOP PLATES (2x MIN.) ³ | D.F. #2, MIN. U.N.O. | 6x POSTS/BEAMS/RAFTERS | D.F. #1, MIN. U.N.O. |
| RIM JOIST (2x DIMENSIONAL) | D.F. #2, MIN. U.N.O. | STRUCTURAL PLYWOOD | APA RATED SHGT |
| FLR JOISTS (2x DIMENSIONAL) | D.F. #2, MIN. U.N.O. | LUMBER EXPOSED TO WEATHER ⁴ | P.T. D.F. #2 U.N.O. |
1. SOLE PLATES, AS NOTED ABOVE, ARE ALL INTERIOR SILL PLATES NOT IN CONTACT WITH CONCRETE. MATERIAL SHALL BE UC1 INTERIOR/DRY CATEGORY AS DEFINED BY AWPA STANDARD U1.
2. MUDSILLS, AS NOTED ABOVE, ARE ALL INTERIOR SILL PLATES IN DIRECT CONTACT WITH CONCRETE. MATERIAL SHALL BE UC2 INTERIOR/DAMP CATEGORY AS DEFINED BY AWPA STANDARD U1.
3. LUMBER EXPOSED TO WEATHER, AS NOTED ABOVE, IS ALL EXTERIOR LUMBER ABOVE GROUND AND EXPOSED TO WEATHER. MATERIAL SHALL BE UC3B ABOVE GROUND EXPOSED CATEGORY AS DEFINED BY AWPA STANDARD U1.
3. **PRESERVATIVE TREATED OR NATURALLY DURABLE MATERIALS:**
LUMBER SHALL BE TREATED WITH TYPICAL WATERBORNE PRESERVATIVES: ALKALINE COPPER QUAT (ACQ-C, ACQ-D, ACQ-D CARBONATE), COPPER AZOLE (CA-B & CA-C, MCA-C) AND SODIUM BORATES (SBX/DOT). THESE TREATMENTS ARE OFTEN REFERRED TO BY TRADE NAMES SUCH AS: WOLMANIZED NATURAL SELECT™ (COPPER AZOLE), PRESERVE AND NATUREWOOD® (ACQ), MICROPRO™, SMART SENSE™ (MCQ), AND ADVANCE GUARD® (BORATE).
- A. ALL INTERIOR/DAMP CONSTRUCTION, UC2 CATEGORY, SHALL BE TREATED WITH SODIUM BORATE SBX/DOT.
1. PLAIN CARBON STEEL FASTENERS, INCLUDING NUTS AND WASHERS, IN CONTACT WITH SODIUM BORATE SBX/DOT PRESERVATIVE-TREATED WOOD IN AN INTERIOR, DRY ENVIRONMENT SHALL BE PERMITTED.
- B. ALL EXTERIOR ABOVE GROUND CONSTRUCTION (INCL. DECK FRAMING), UC3B CATEGORY, SHALL BE TREATED WITH ALKALINE COPPER QUAT (ACQ-C, ACQ-D, ACQ-D CARBONATE), OR COPPER AZOLE (CA-B & CA-C, MCA-C)
1. ALL FASTENERS AND CONNECTORS IN CONTACT WITH PRESERVATIVE-TREATED TREATED WOOD USING ALKALINE COPPER QUAT (ACQ-C, ACQ-D, ACQ-D CARBONATE), OR COPPER AZOLE (CA-B & CA-C, MCA-C) SHALL BE HOT DIPPED GALVANIZED OR STAINLESS STEEL IN ACCORDANCE WITH GOVERNING CBC SEC. 2304.10 AND SHALL BE IN ACCORDANCE WITH ASTM A 153.
- C. WOOD JOISTS OR WOOD STRUCTURAL FLOORS THAT ARE CLOSER THAN 18 INCHES OR WOOD GIRDERS THAT ARE CLOSER THAN 12 INCHES TO EXPOSED GROUND IN CRAWL SPACES OR UNEXCAVATED AREAS LOCATED WITHIN THE PERIMETER OF THE BUILDING FOUNDATION SHALL BE PRESERVATIVE-TREATED WOOD IN ACCORDANCE CBC.
4. **GLUE-LAMINATED STRUCTURAL MATERIALS:**
STANDARD SPECIFICATIONS FOR GLUE-LAMINATED STRUCTURAL MEMBERS, ANSI/AITC A 190.1 AND ASTM D3737. GLUE-LAMINATED BEAMS SHALL BE INSPECTED AND A CERTIFICATE PROVIDED TO THE FILED INSPECTOR AT THE TIME OF FRAMING INSPECTION. FABRICATION SHALL BE PERFORMED IN ACCORDANCE WITH CBC 1705.5. ALL GLUE-LAMINATED BEAMS THAT ARE CONTINUOUS OVER SUPPORTS OR CANTILEVERED SHALL HAVE TENSION LAMINATIONS ON TOP OF BEAMS.
MAXIMUM MOISTURE CONTENT SHALL NOT EXCEED 16% AT THE TIME OF CONSTRUCTION.
- A. GLULAM MEMBERS IN DRY SERVICE USE SHALL BE DOUGLAS-FIR 24F-V4 (SINGLE SPAN) OR COMBINATION 24F-V8 (MULTI-SPAN & CANTILEVER) U.N.O. BEAMS SHALL BE ARCHITECTURAL GRADE WHEN EXPOSED TO VIEW, S.A.D.
1. GLULAM MEMBERS, SEE PLAN SPECIFICATIONS FOR CAMBER WHERE OCCURS.
- | MATERIAL GRADE | E (x10 ³ psi) | Fb (psi) | Fv (psi) | Fc (psi) | RADIUS, U.N.O. |
|----------------|--------------------------|----------|----------|----------|----------------|
| DF/DF 24F-V4 | 1.8 | 2400 | 265 | 650 | 3500' RADIUS |
| DF/DF 24F-V8 | 1.8 | 2400 | 265 | 650 | NO CAMBER |
- B. GLULAM MEMBERS IN WET SERVICE USE SHALL BE ALASKAN CEDAR 20F-V12 (SINGLE SPAN) OR COMBINATION 20F-V13 (MULTI-SPAN & CANTILEVER) U.N.O.
1. GLULAM MEMBERS, SEE PLAN SPECIFICATIONS FOR CAMBER WHERE OCCURS.
- | MATERIAL GRADE | E (x10 ³ psi) | Fb (psi) | Fv (psi) | Fc (psi) | RADIUS, U.N.O. |
|----------------|--------------------------|----------|----------|----------|----------------|
| AC/AC 20F-V12 | 1.5 | 2000 | 265 | 560 | 3500' RADIUS |
| AC/AC 20F-V13 | 1.5 | 2000 | 265 | 560 | NO CAMBER |
- C. DESIGN, FABRICATION AND CONSTRUCTION OF STRUCTURAL GLULAM MEMBERS SHALL CONFORM TO THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION STANDARD, MANUAL No. 301 AND THE COMMERCIAL STANDARD FOR STRUCTURAL GLUED LAMINATED TIMBER, CS 253.
- D. STRUCTURAL PLYWOOD SHALL BE GRADED PER APA PSI-83AND SHALL BE INTERIOR TYPE SHEATHING C-D GRADE WITH EXTERIOR GLUE. EQUIVALENT O.S.B. WOOD STRUCTURAL PANEL MAY BE USED AS AN ALTERNATE TO PLYWOOD. HOWEVER, IN ACCORDANCE WITH THE TILE COUNCIL OF AMERICA RECOMMENDATIONS, O.S.B. SHALL NOT BE USED BELOW TILE MORTAR. ALL HORIZONTAL PLYWOOD SHALL BE LAID WITH FACE GRAIN PERPENDICULAR TO FRAMING MEMBERS, WITH STAGGERED JOINTS.
5. **STRUCTURAL COMPOSITE LUMBER (SCL) MATERIALS:**
ALL STRUCTURAL COMPOSITE LUMBER (SCL) SHALL BE MANUFACTURED PER ASTM D5055 AND ASTM D5456. STRUCTURAL COMPOSITE LUMBER SHALL BE MANUFACTURED BY BOISE CASCADE, OR EQUAL. ALTERNATE MANUFACTURERS ARE PERMITTED AND CONSIDERED AN EQUIVALENT SUBSTITUTION IF THE SUBSTITUTED MATERIAL MEETS, OR EXCEEDS, ALL THE DESIGN PROPERTIES LISTED BELOW.
- | SPECIFICATION | MATERIAL | WIDTH (in.) ² | E (x10 ³ psi) | Fb (psi) | Fv (psi) | Fc ₁ (psi) | Fc ₂ (psi) |
|---------------|------------------|--------------------------|--------------------------|----------|----------|-----------------------|-----------------------|
| RIM JOIST | VERSA-STRAND LVL | 1-5/16 | 1.4 | 1800 | 225 | 525 | 2500 |
| LVL COLUMN | VERSA-LAM LVL | 3.5, 5, 25, 7 | 1.7 | 2650 | 285 | 750 | 3000 |
| LVL STUD | VERSA-STUD LVL | 1.5 | 1.7 | 2617 | 285 | 450 | 3000 |
| LVL BEAM | VERSA-LAM 2.0E | 1.75 | 2.0 | 2800 | 285 | 750 | 3000 |
| LVL BEAM | VERSA-LAM 2.0E | 3.5, 5, 25, 7 | 2.0 | 3100 | 285 | 750 | 3000 |
1. BEAM DEPTH IS ASSUMED TO MATCH FLOOR FRAMING DEPTH U.N.O.
2. MFR BEAM MEMBERS MAY BE BUILT UP TO ACHIEVE SPECIFIED WIDTHS PER DETAIL 609 SD6 U.N.O. ON PLANS.
6. ALL STUD WALLS SHOWN ON STRUCTURAL DRAWINGS SHALL BE FRAMED AS FOLLOWS (U.O.N.):
- EXTERIOR WALL STUD HEIGHT TABLE (L/360 DEFL.)
- | MAX. PLATE HT | STUD TYPE | SPACING |
|---------------|---------------------|----------|
| 21'-5" | (2) 2x6 DOUG-FIR #1 | 12" O.C. |
| 19'-5" | (2)-2x6 DOUG-FIR #1 | 16" O.C. |
| 15'-1" | 2x6 DOUG-FIR #2 | 16" O.C. |
| 12'-1" | (2)-2x4 DOUG-FIR #1 | 16" O.C. |
| | 2x6 STUD GRADE | 16" O.C. |
| | 2x4 DOUG-FIR #1 | 16" O.C. |
| 10'-1" | 2x6 STUD GRADE | 16" O.C. |
| 9'-1" | 2x4 DOUG-FIR #2 | 16" O.C. |
| | 2x6 STUD GRADE | 16" O.C. |
| 8'-1" | 2x4 STUD GRADE | 16" O.C. |
| | 2x6 STUD GRADE | 16" O.C. |
- INTERIOR WALL STUD HEIGHT TABLE (L/240 DEFL.)
- | MAX. PLATE HT | STUD TYPE | SPACING |
|---------------|----------------|----------|
| 21'-5" | 2x6 STUD GRADE | 12" O.C. |
| 19'-5" | 2x6 STUD GRADE | 16" O.C. |
| 15'-1" | 2x6 STUD GRADE | 16" O.C. |
| 12'-1" | 2x4 STUD GRADE | 16" O.C. |
| | 2x6 STUD GRADE | 16" O.C. |
- NOTE:
- A. STUD HEIGHTS EXCEEDING 10'-1" SHALL BE BRACED MID-HEIGHT USING FULL DEPTH 2x BLOCKING.
7. TOP PLATES SHALL BE DOUBLED ON ALL STUD WALLS. LAP 4'-0" MINIMUM AT TOP PLATE SPLICES, WITH (12) 16d NAILS EACH SIDE OF SPLICE, U.N.O. SPLICES IN UPPER AND LOWER PLATES SHALL BE STAGGERED 4'-0" MINIMUM.
8. POSTS IN WALLS MAY BE MADE WITH MULTIPLE STUDS OF EQUIVALENT WIDTH AND DEPTH, U.O.N. SECURE MULTIPLE STUDS WITH 16d NAILS AT 8" O.C. STAGGERED.
9. PROVIDE KING STUDS AT THE ENDS OF ALL HEADERS OR OTHER BEAMS INSTALLED IN WALLS, SEE DETAIL 401 SD4 ADJACENT, STACKING WINDOWS SHALL BE SEPARATED BY KING STUDS THAT ARE CONTINUOUS FROM SILL TO TOP PLATE, CRIPPLE STUDS UNDER HEADERS SHALL BE CONTINUOUS TO SILL PLATE. END NAIL KING STUDS TO HEADERS.
10. ALL MEMBERS IN BEARING SHALL BE ACCURATELY CUT AND ALIGNED SO THAT FULL BEARING IS PROVIDED WITHOUT THE USE OF SHIMS.
11. BLOCK ALL STUD WALLS AS REQUIRED FOR SHEATHING AND FINISHES. BALLOON FRAME ALL WALLS WITH SLOPING CEILING OR WITH RAISED CEILINGS.
12. INSTALL HORIZONTAL MEMBERS WITH CROWN UP. WHERE KNOTS EXIST NEAR THE TOP OR BOTTOM OF HORIZONTAL MEMBERS, INSTALL MEMBER WITH KNOTS UP. CANTILEVERED DECK JOISTS SHALL BE CAREFULLY NOTCHED AND TRIMMED (IF NECESSARY) TO PROVIDE SLOPE WITHOUT OVER-CUTTING.
13. PROVIDE FULL DEPTH BLOCKING OR CONTINUOUS RIM JOIST AT ALL FLOOR AND ROOF FRAMING SUPPORTS. FRAMING MEMBERS SHALL HAVE A MINIMUM OF 2" BEARING AT SUPPORTS. LAPPING JOISTS SHALL HAVE 6" MINIMUM OVERLAP CENTERED OVER INTERIOR SUPPORTS.
14. ALL BOLTED WOOD CONNECTIONS SHALL HAVE A WASHER UNLESS A STEEL PLATE IS SPECIFIED. HOLES SHALL BE PROPERLY ALIGNED. OVERSIZED HOLES ARE NOT ALLOWED. NUTS SHALL BE SNUG TIGHTENED. BOLT HOLES SHALL BE NOMINAL DIAMETER OF BOLT PLUS 1/16".
15. NAILED WOOD CONNECTIONS SHALL USE COMMON WIRE NAILS, U.N.O. MIN. NAILING REQUIREMENTS FOR STANDARD CONNECTIONS SHALL BE IN ACCORDANCE WITH THE GOVERNING C.B.C. FASTENING SCHEDULE (TABLE 2304.10.1).
16. ALL MANUFACTURED CONNECTION HARDWARE SHALL BE AS DESIGNATED ON DRAWINGS AND INSTALLED (WITH ALL NAIL HOLES FILLED) IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS AND APPLICABLE ICC APPROVALS.
17. INSTALL LAG SCREWS IN DRILLED LEAD HOLES WITH A DIA. EQUAL TO 3/4" OF THE SHANK DIAMETER. LAG SCREWS SHALL NOT BE HAMMERED IN. PROVIDE WASHERS UNDER HEADS BEARING ON WOOD. HOLES SHALL BE PROPERLY ALIGNED.

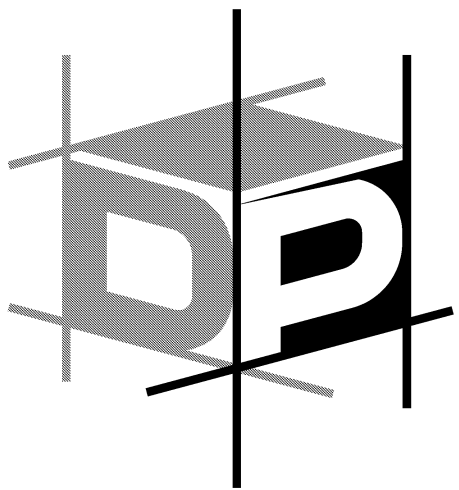
CONCRETE NOTES

- CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF THE 2022 BUILDING CODE (C.B.C.) AND THE LATEST EDITION OF THE AC1318-14, UNLESS NOTED OTHERWISE.
1. CONCRETE REQUIREMENTS: BASED ON NORMAL WEIGHT CONCRETE (UNIT WEIGHT OF 145 TO 150 pcf).
- | | MINIMUM COMPRESSIVE STRENGTH (psi) (AT 28 DAYS) | SLUMP (+/- 1/2") | AGGREG. SIZE | SPEC. INSP. REQD | MAXIMUM WATER TO CEMENT RATIO (NOTE 1F) | CEMENT TYPE ASTM C150 |
|--------------------------|---|------------------|--------------|------------------|---|-----------------------|
| GRADE BEAMS & STEM WALLS | 3000 | 4" | 3/4" | NO ** | .55 | II OR V |
| CONCRETE PIERS | 3000 | 4" | 3/4" | NO ** | .55 | II OR V |
| FOOTINGS ON GRADE | 3000 | 4" | 1" | NO ** | .55 | II OR V |
| FLOOR SLABS ON GRADE | 3000 | 4" | 1" | NO ** | .45 | II OR V |
- ** **SPECIAL INSPECTION IS NOT REQUIRED, DESIGN COMPRESSIVE STRENGTH IS 2500 psi, HIGHER STRENGTHS HAVE BEEN SPECIFIED FOR QUALITY CONTROL.**
- A. COARSE AGGREGATE SHALL BE HARD, DURABLE CRUSHED STONE OR GRAVEL GRADED PER ASTM C33. MAXIMUM SIZE OF AGGREGATE SHALL BE AS NOTED IN SCHEDULE ABOVE AND DEFINED BELOW. SAND SHALL BE CLEAN, HARD, DURABLE, WASHED FREE FROM SILT, LOAM OR CLAY.
1. GRADE BEAMS, STEM WALLS, RETAINING WALLS AND OTHER CONCRETE FRAMEWORK LESS THAN 10" WIDE SHALL USE 3/4" AGGREGATE CONFORMING TO ASTM C33 WITH 100% PASSING 1" SIEVE AND 90% (MINIMUM) PASSING 3/4" SIEVE. TO ENSURE PROPER CONCRETE COVER AND CONSOLIDATION.
2. FOOTING AND SLABS ON GRADE, INCLUDING POST-TENSION AND MAT SLABS, SHALL USE 1" AGGREGATE CONFORMING TO ASTM C33 WITH 95% (MINIMUM) PASSING 1" SIEVE.
- B. MIXING WATER SHALL BE CLEAN AND FREE FROM INJURIOUS AMOUNTS OF OIL, ACIDS, ALKALIES, ORGANIC MATERIALS OR OTHER DELETERIOUS SUBSTANCES.
- C. CONCRETE EXPOSED TO SULFATE SHALL USE TYPE V CEMENT WITH POZZOLAN.
- D. FLY ASH OR POZZOLANS, IF USED, SHALL CONFORM WITH ASTM C618, COAL FLY ASH AND RAW OR CALCINED NATURAL POZZOLAN FOR USE AS A MINERAL ADMIXTURE IN CONCRETE. USAGE SHALL NOT EXCEED 25 PERCENT, BY WEIGHT OF THE TOTAL CEMENTITIOUS MATERIALS. WHEN POZZOLANS ARE USED TO MITIGATE THE EFFECT OF SULFATE CONTAINING SOILS THEY SHALL BE OF A TYPE THAT HAS DEMONSTRATED SUCH ABILITY BY TEST OR SERVICE RECORD.
- E. ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL NOT BE USED.
- F. CEMENT SHALL CONFORM WITH ASTM C150 & C 595, PORTLAND CEMENT AND BLENDED HYDRAULIC CEMENTS
- G. TRANSIT MIX SHALL BE PER ASTM C-94.
- H. MIX DESIGN SHALL MEET THE RECOMMENDED SPECIFICATION UNLESS AN ALTERNATE MIX IS SUBMITTED AND REVIEWED BY THE ENGINEER.
2. SPECIAL INSPECTION OF CONCRETE WORK IS REQD WHERE NOTED ABOVE. WHEN REQD, SPECIAL INSPECTION SHALL INCLUDE THE INSPECTION OF THE PLACEMENT OF REINFORCEMENT, AND THE INSPECTION OF THE CONCRETE PLACEMENT OPERATIONS AS WELL AS CONCRETE CYLINDER TESTS, PER C.B.C. SECTION 1705.3. THE FOLLOWING THREE CRITERIA ESTABLISH THE REQD MINIMUM SAMPLING FREQUENCY FOR EACH CLASS OF CONCRETE:
- A. ONE EACH DAY A GIVEN CLASS IS PLACED, NOR LESS THAN
- B. ONCE FOR EACH 150 yd OF EACH CLASS PLACED EACH DAY, NOR LESS THAN
- C. ONCE FOR EACH 5000 ft OF SLAB OR WALL SURFACE AREA PLACED EACH DAY.
3. SPLICES OF CONTINUOUS REINFORCEMENT SHALL HAVE A MINIMUM LAP PER DETAILS 202 SD2 AND 203 SD2 UNLESS NOTED OTHERWISE. ALL REINFORCING STEEL SHALL BE SECURELY WIRED AND PROPERLY SUPPORTED ABOVE GROUND, AND AWAY FROM FORMS. REINFORCING BAR FABRICATION, LAPS AND PLACEMENT SHALL CONFORM TO THE MANUAL OF STANDARD PRACTICE OF THE CONCRETE REINFORCING STEEL INSTITUTE.
4. REINFORCING SHALL BE NEW STOCK, DEFORMED BARS, NO. 3 AND SMALLER: GRADE 40 CONFORMING TO ASTM A-615 AS FOLLOWS (U.N.O.) : NO. 4 AND LARGER: GRADE 60
- A. ALL BARS TO BE WELDED SHALL MEET THE REQUIREMENTS OF ASTM A706, GRADE 60.
- B. WELDED WIRE FABRIC SHALL CONFORM TO ASTM
- A-185, FLAT SHEETS ONLY. LAP FABRIC 6" MINIMUM.
- C. REINFORCEMENT PLACEMENT SHALL CONFORM TO DETAIL
- D. ANCHOR BOLTS SHALL BE ASTM A307, U.N.O. AND SHALL CONFORM TO DETAIL 201 SD2
5. REINFORCEMENT COVER: ALL DIMENSIONS SHOWING THE LOCATIONS OF REINFORCEMENT STEEL NOT NOTED AS "CLEAR", ARE TO THE CENTER OF THE STEEL. MINIMUM CLEAR COVERAGE OF REINFORCEMENT SHALL BE AS FOLLOWS:
- A. CONCRETE CAST AGAINST EARTH, EXCEPT SLABS ON GRADE: 3" 1-1/2" U.N.O.
- SLABS ON GRADE: 1-1/2" U.N.O.
- B. CONCRETE CAST IN FORMS, BUT EXPOSED TO EARTH OR WEATHER: No. 5 REINFORCING AND SMALLER: 1-1/2" No. 6 REINFORCING AND LARGER: 2"
- C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND: SLABS, WALLS AND JOISTS: 3/4" BEAMS AND COLUMNS: 1-1/2"
6. UNFORMED CONCRETE SURFACE CURING:
- A. CURE FOR ONE TO SEVEN DAYS BY MAINTAINING TEMPERATURE ABOVE 50 DEGREES FAHRENHEIT, AND IN A MOIST CONDITION.
- B. APPLY MEMBRANE-FORMING CURING COMPOUND TO DAMP CONCRETE IMMEDIATELY AFTER COMPLETION OF THE MOIST-CURING PERIOD.
7. THE CONTRACTOR SHALL INFORM THE ENGINEER AT LEAST TWO DAYS PRIOR TO POURING ANY STRUCTURAL CONCRETE SO THAT OBSERVATION OF THE WORK MAY BE PERFORMED AS REQUIRED BY THE ENGINEER'S CONTRACT OR THE CODE.
8. FOOTING/GRADE BEAM CONSTRUCTION JOINTS SHALL CONFORM TO DETAIL 204 SD2
9. CRACK CONTROL JOINTS SHALL BE PLACED IN CONCRETE SLABS ON GRADE AT A SPACING OF 12'-0" MAX. O.C. EACH WAY (U.N.O. ON PLAN) PER DETAIL 205A SD2
- LOCATION OF SLAB ON GRADE CONSTRUCTION JOINTS SHALL BE APPROVED BY THE ENGINEER PRIOR TO CONSTRUCTION. SLAB ON GRADE CONSTRUCTION JOINTS SHALL CONFORM TO 205B SD2
10. ALL PIPES AND DUCTS THROUGH CONCRETE SHALL BE SLEEVED. VERIFY OPENINGS WITH PLUMBER AND ELECTRICIAN. SEE DETAIL 206 SD2
11. IF SPECIFIED, WELDED WIRE FABRIC SHALL BE 6x6-W1.4xW1.4. WIRE FABRIC SHALL BE ELECTRICALLY WELDED STEEL PER ASTM A185. LAP 6" MINIMUM AT ALL EDGES AND TIE AT THREE PLACES TO REINFORCING DOWELS (WHERE OCCUR) EXCEPT LOCATIONS WHERE SLAB IS INDEPENDENT OF FOUNDATION. CONTRACTOR SHALL PROVIDE SUPPORT CHAIRS TO ENSURE FABRIC IS LOCATED IN THE CENTER OF THE SLAB.
12. WELDING OF REINFORCING BARS SHALL CONFORM TO AWS D1.4 USING ASTM A706 REINFORCING BAR SPECIFICATIONS.

STRUCTURAL STEEL NOTES

1. THE FOLLOWING SECTION APPLIES TO ALL STRUCTURAL STEEL 1/8" THICK OR LARGER.
2. ALL WORK SHALL BE IN ACCORDANCE WITH THE CBC CHAPTER 22, AISC 15th EDITION, AND THE 2015 A.W.S. D1.1.
3. STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING:
- A. PLATES AND SHAPES, U.N.O. ASTM A992 (fy = 50 ksi), OR ASTM A572, GRADE 50
- B. STRUCTURAL TUBE COLUMNS AND BEAMS: ASTM A500, GRADE B.
- C. PIPE COLUMNS: ASTM A53, TYPE E OR S, GRADE B
- D. ROD ASTM A36 (fy = 36 ksi)
4. FASTENERS SHALL CONFORM TO THE FOLLOWING:
- A. ANCHOR BOLTS: ASTM A307, U.N.O.
- B. STEEL TO WOOD CONNECTIONS: ASTM A307, U.N.O. USE CUT WASHERS (IF CONCEALED) AND MALLEABLE IRON WASHERS (IF EXPOSED)
5. WELDING ELECTRODES SHALL CONFORM TO 2015 A.W.S. D1.1 AND SHALL BE LOW HYDROGEN MATCHING FILLER METAL.
6. SURFACES OF STEEL TO BE FIELD WELDED SHALL BE FREE AND CLEAR OF ALL PAINT, DIRT, GREASE, OR OTHER DELETERIOUS COATINGS.
7. STEEL FRAMING, EXCEPT THOSE PORTIONS TO BE EMBEDDED IN CONCRETE, CONCEALED IN FRAMING, FIELD WELDED, OR HIGH STRENGTH BOLTED SHALL BE SHOP-PAINTED PER THE SPECIFICATIONS.
8. FIELD PAINT ALL EXPOSED STEEL SURFACES AFTER INSTALLATION, PER THE SPECIFICATIONS.
9. ALL WELDERS SHALL BE QUALIFIED BY A.W.S. PROCEDURES FOR THE REQUIRED WELDING.
10. SUBMIT CERTIFICATION OF COMPLIANCE FOR ALL STEEL MATERIALS.

DO NOT SCALE THESE DRAWINGS
PLAN SET ID: 1-ME-RP



ADVANCED
ENGINEERING

3381 Walnut Blvd. Ste. 220
Brentwood, CA 94513
Office: 925.516.3502
Fax: 925.262.4662

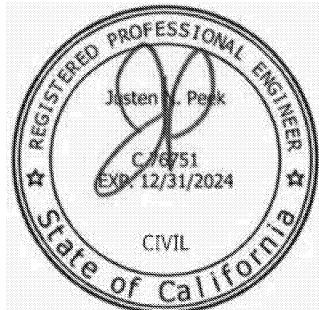
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Engineering Seal:



Sheet Description:

GENERAL STRUCTURAL
SPECIFICATIONS AND NOTES

No Scale

Initial Issue Date: March 24, 2023

Drawn By: E. Bennett

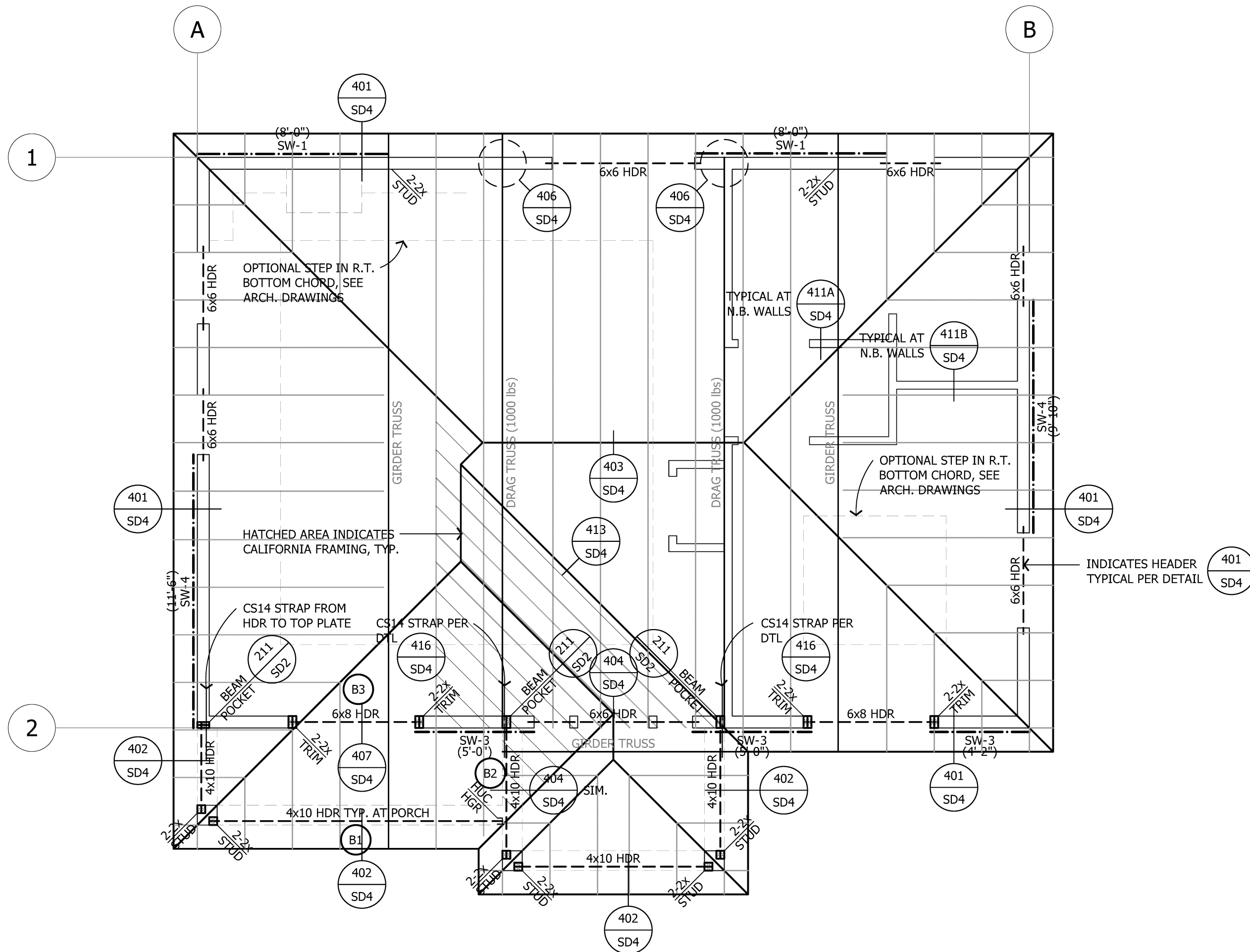
Project Designer: J. Peek

Project Manager: J. Peek Ext. 23

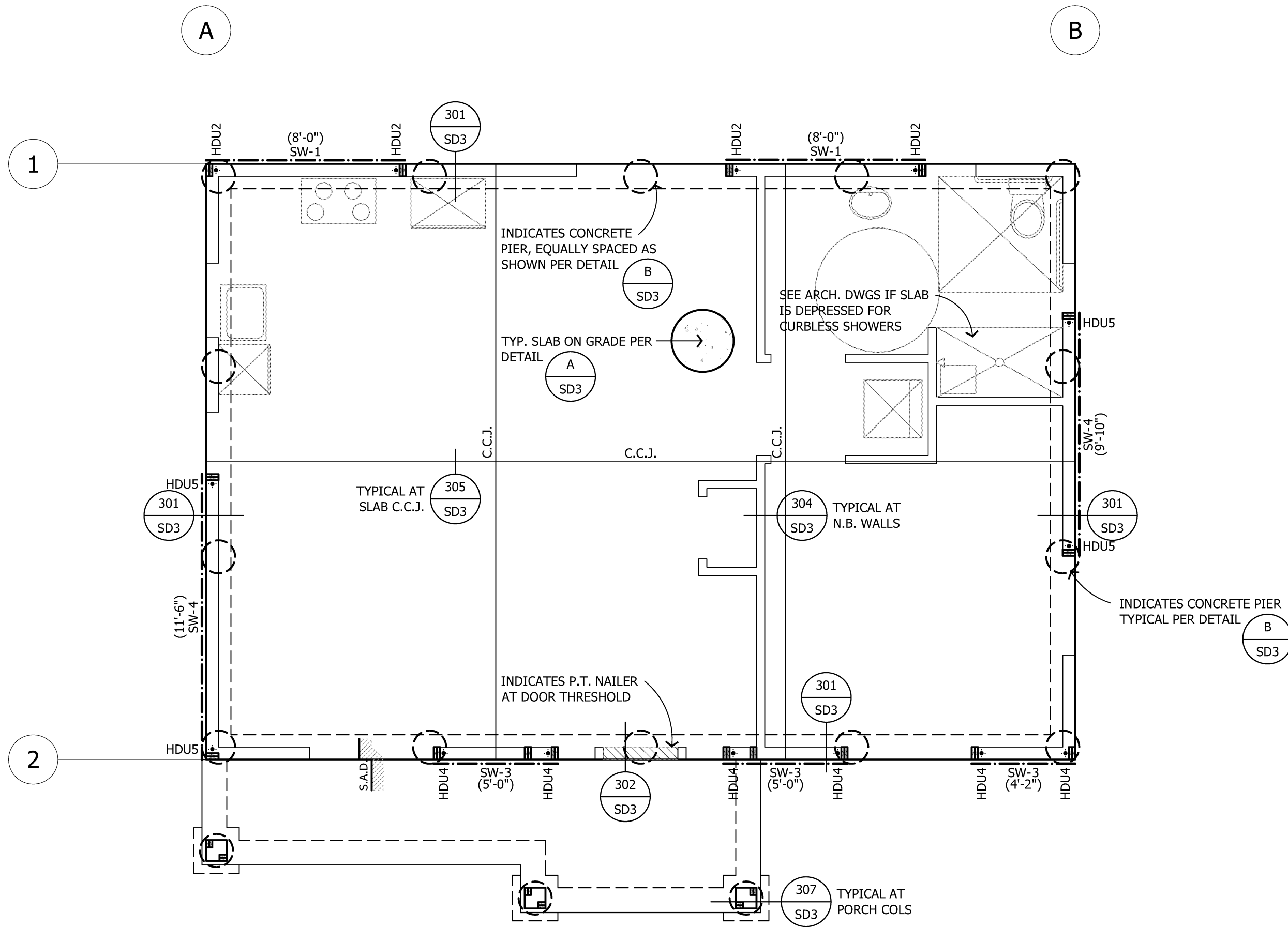
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A ROOF FRAMING PLAN - MEDITERRANEAN



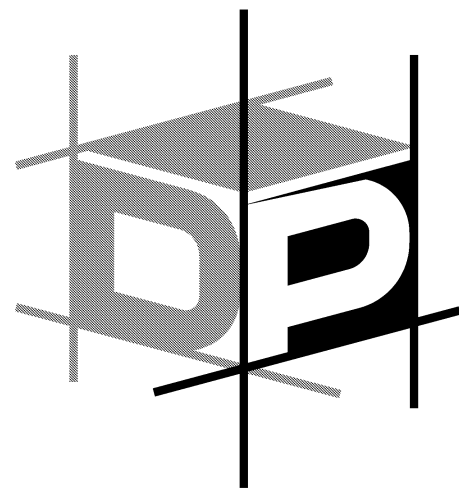
B FOUNDATION PLAN - MEDITERRANEAN - CONCRETE SLAB ON PIERS

FOUNDATION NOTES

- SEE ARCHITECTURAL DRAWINGS FOR STUD SIZES AND OTHER REQUIREMENTS.
- SEE ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS, ELEVATIONS, ETC. THE PROJECT ARCHITECT IS RESPONSIBLE FOR SPECIFYING DIMENSIONS TO ALL PROJECT ELEMENTS. THE DIMENSIONS SHOWN ON THE FOUNDATION PLANS SHALL NOT BE USED FOR CONSTRUCTION. THE FOUNDATION SHALL BE CONSTRUCTED USING THE DIMENSIONS SHOWN ON THE ARCHITECTURAL PLANS.
- DETAIL KEYS AND OTHER INFORMATION ARE TYPICAL.
- THE FOUNDATION SHALL CONSIST OF A UNIFORM THICKNESS REINFORCED CONCRETE STRUCTURAL SLAB PER DETAIL (B/SD3)
 - STEP FOUNDATION AS (A/SD3) REQUIRED FOR FLOOR ELEVATION CHANGES AND COMPLIANCE WITH DETAILS. THE CONTRACTOR SHALL GRADE THE SITE TO PROVIDE PROPER SURFACE DRAINAGE AWAY FROM THE FOUNDATION AT ALL LOCATIONS. ROOF GUTTER DOWNSPOUTS SHALL NOT DISCHARGE NEAR THE FOUNDATION.
- CONCRETE TIE BEAMS SHALL BE 12" x 18" AND SHALL BE REINFORCED WITH (2) #4 HORIZ. REINFORCING, TOP AND BOTTOM. TIE BEAM REINFORCEMENT SHALL CONTINUE THROUGH FOUNDATION ELEMENTS OR SHALL BE PROPERLY TIED TO FOUNDATION REINFORCING.
- LANDSCAPE SLABS SHALL BE INDEPENDENT OF THE FOUNDATION. THE CONTRACTOR SHALL INSTALL ASPHALT-IMPREGNATED EXPANSIVE JOINT MATERIAL BETWEEN SLABS AND THE FOUNDATION.
- HOLDOWNS:
HOLDOWNS NOTED OCCUR AT THE LEVEL OF FRAMING SHOWN FOR CONNECTION OF WALLS ABOVE TO FOOTING.
 - ALL HOLDOWN MEMBERS IN CONTACT WITH P.T. MATERIAL SHALL BE HOT-DIP GALVANIZED. USE HOT-DIPPED GALVANIZED ANCHOR BOLTS WITH 3" x 3" x 1/4" PLATE WASHERS. PLAIN CARBON STEEL FASTENERS IN SBX/DOT AND ZINC BORATE PRESERVATIVE-TREATED WOOD SHALL BE PERMITTED.
 - ALL HOLDOWNS SHALL BE INSTALLED WITH ANCHORS PER DETAIL (112/SD1)
 - CONTRACTOR SHALL REVIEW HOLDOWN ANCHOR REQUIREMENTS FOR EMBEDMENT CONDITIONS THAT REQUIRE DEEPENED STEM WALL. REFER TO FOR REQUIREMENTS.
 - PROVIDE POSTS AT ALL HOLDOWNS AS SHOWN IN DETAIL (112/SD1) WHERE DOUBLE 2x POSTS ARE USED, SISTER WITH 16d FACE NAILS AT SILL NAILING PER SHEARWALL SCHEDULE. WHERE 3x MEMBERS AT PANEL EDGES ARE REQUIRED, USE 4x MINIMUM POST FOR HOLDOWN PER FRAMER SHALL CONFIRM LAYOUT BEFORE CONCRETE IS POURED. NOTIFY ENGINEER OF CONFLICTS PRIOR TO PROCEEDING.
- CONTRACTOR SHALL REVIEW HOLDOWN ANCHOR REQUIREMENTS FOR EMBEDMENT CONDITIONS THAT REQUIRE DEEPENED FTG. REFER TO FOR REQUIREMENTS. (112/SD1)
- REFER TO THE PROJECT SOILS REPORT IF APPLICABLE, FOR ADDITIONAL FOUNDATION AND SITE CONSTRUCTION REQUIREMENTS. ALL FOUNDATION ELEMENTS SHALL COMPLY WITH ALL CONDITIONS CONTAINED WITHIN APPROPRIATE SOILS REPORT AND REQUIREMENTS OF 2016 CBC CHAPTER 18. THE RESPONSIBLE SOILS ENGINEER IF APPLICABLE, SHALL OBSERVE ALL SLAB AND FOUNDATION SUBGRADES PRIOR TO PLACING CONCRETE. SEE SOILS REPORT FOR SPECIFIC INSPECTION REQUIREMENTS.
- ALL FOUNDATION PLATES AND MUDSILLS SHALL BE PRESSURE-TREATED DOUGLAS FIR MARKED OR BRANDED BY AN APPROVED AGENCY. SEE SHEARWALL SCHEDULE FOR SHEARWALL LOCATIONS THAT REQUIRE 3x MINIMUM MUDSILLS. FOUNDATION PLATES AND MUDSILLS SHALL BE BOLTED TO THE FOUNDATION WITH NOT LESS THAN 5/8" DIA. HOT-DIPPED GALVANIZED ANCHOR BOLTS WITH 3" x 3" x 1/4" PLATE WASHERS. PLAIN CARBON STEEL FASTENERS IN SBX/DOT AND ZINC BORATE PRESERVATIVE-TREATED WOOD SHALL BE PERMITTED. EMBED ANCHOR BOLTS 7" INTO CONCRETE FOUNDATION, WALL OR 12" INTO GROUTED MASONRY, AND SPACE NOT MORE THAN 4'-0" APART, UNLESS NOTED OTHERWISE ON THE SHEARWALL SCHEDULE. THERE SHALL BE A MINIMUM OF TWO BOLTS PER BOARD WITH ONE BOLT LOCATED 12" MAXIMUM AND 4-3/8" MINIMUM OF EACH END. FOR ADDITIONAL INFORMATION SEE DETAIL (201/SD2)
- SEE GENERAL NOTES FOR ADDITIONAL REQUIREMENTS. ALL DETAIL CALLOUTS SHALL BE CONSIDERED TYPICAL. CONTRACTOR SHALL REVIEW THE DETAIL SHEETS FOR DETAILS NOT SPECIFICALLY REFERENCED.
- SHEARWALLS:
LENGTHS OF SHEARWALLS ARE SHOWN ABOVE OR BELOW SHEARWALL NUMBER (SEE PLAN). THE NUMBERS INDICATE THE MINIMUM DESIGN LENGTH OF THE SHEARWALL. SEE DETAIL (103/SD1)

ROOF FRAMING NOTES

- WALLS SHOWN AS SOLID ARE BELOW FRAMING. SEE ARCHITECTURAL DRAWINGS FOR STUD SIZES AND OTHER REQUIREMENTS.
- SEE ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS, ELEVATIONS, ETC. THE PROJECT ARCHITECT IS RESPONSIBLE FOR SPECIFYING DIMENSIONS TO ALL PROJECT ELEMENTS.
- DETAIL KEYS AND OTHER INFORMATION ARE TYPICAL.
- ROOF FRAMING TERMS:
ALL BEAMS SHALL BE CONSIDERED FLUSH, (TOP OF BEAM EQUAL TO TOP OF ROOF FRAMING AND BOTTOM OF ROOF FRAMING) UNLESS NOTED OTHERWISE ON PLANS AS ONE OF THE FOLLOWING:
 - DROPPED:** BEAM IS SET BELOW ROOF FRAMING WITH TOP OF BEAM SET AT BOTTOM OF ROOF FRAMING.
 - DOWNSET:** BEAM IS DEEPER THAN THE ROOF FRAMING AND SET WITH TOP OF BEAM EQUAL TO TOP OF ROOF FRAMING. BOTTOM OF BEAM EXTENDS BELOW ROOF FRAMING.
 - UPSET:** BEAM IS DEEPER THAN THE ROOF FRAMING AND SET WITH BOTTOM OF BEAM EQUAL TO BOTTOM OF ROOF FRAMING. TOP OF BEAM EXTENDS ABOVE ROOF FRAMING.
 - "HDR":** BOTTOM OF HEADER IS SET AT ROUGH OPENING (R.O.). SEE ARCH. FOR HEIGHT. IF HEADER BREAKS TOP PLATES INSTALL CS14 x 36" STRAP FROM FACE OF PLATE TO FACE OF HEADER, TYPICAL AT EACH END OF HEADER.
- ROOF FRAMING:
ROOF SHALL BE FRAMED WITH PRE-MANUFACTURED ROOF TRUSSES AT 24" O.C. INSTALLED PER APPROVED ROOF TRUSS SHOP DRAWINGS. ROOF TRUSS MANUFACTURER SHALL DESIGN THE ROOF TRUSSES AS SPECIFIED IN THE GENERAL NOTES (SHEET SN-2, U.N.O.). SEE ROOF TRUSS DETAILS DETAILS ON SHEET SD4 FOR TYPICAL DETAILING.
 - ROOF TRUSSES WITH FLAT BOTTOM CHORDS SHALL BE SECURED TO END BEARING POINTS WITH "H1" CLIP TO TOP PLATE. ROOF TRUSSES WITH SLOPED BOTTOM CHORDS SHALL BE SECURED TO BEARING POINTS WITH SIMPSON "TTC" CLIPS OR EQUIVALENT CONNECTORS THAT ALLOW HORIZONTAL MOVEMENT IN ACCORDANCE WITH THE ROOF TRUSS DESIGNER'S REQUIREMENTS.
 - PROVIDE 2x BLOCKING BELOW ALL HIPs, VALLEYS, AND RIDGES. CONTRACTOR SHALL INSTALL STRONGBACKS, CROSS BRIDGING, AND/OR BRACING AS SPECIFIED BY THE ROOF TRUSS SHOP DRAWINGS.
- MANUFACTURED BEAM FRAMING:**
ALL MANUFACTURED WOOD BEAMS SHALL BE MANUFACTURED BY "TRUSS JOIST" WEYERHAUSER. REFER TO "GENERAL FRAMING NOTES" ON SHEET SN1 FOR MATERIAL SPECIFICATIONS.
- ROOF SHEATHING:**
TYPICAL ROOF SHEATHING SHALL BE 15/32" THICK (24/0 OR 32/16) CDX GRADE PLYWOOD (OR EQUIVALENT-RATED ORIENTED STRAND BOARD (O.S.B.) WITH GAP PER MANUFACTURER. ROOF SHEATHING SHALL BE NAILED WITH 8d AT 6" O.C. (EDGES) AND 8d AT 12" O.C. (FIELD), UNLESS NOTED OTHERWISE ON THE PLAN. ORIENT SHEATHING WITH FACE GRAIN PERPENDICULAR TO FRAMING MEMBERS, AND STAGGER END JOINTS.
 - PLYWOOD SHEETS LOCATED AT ROOF EDGES OR CHANGES IN FRAMING SHALL BE AT LEAST 24 INCHES WIDE OR SHALL BE EDGE LOCKED AND NAILED. EDGE NAIL PLYWOOD TO ALL GABLE AND SHEAR TRUSSES (TRUSSES WITH DRAG LOADS).
 - EDGE NAIL ROOF PLYWOOD TO ALL DRAG MEMBERS (RAFTERS OR BEAMS w/ HORIZ. STRAPS).
- SUPPORT OF BEAMS, HIPs, GIRDERS, ETC.:**
INSTALL POSTS (POST SIZE TO MATCH BEAM AND WALL SIZE), MINIMUM BELOW ALL ROOF BEAMS AND GIRDERS. CONTRACTOR SHALL VERIFY FRAMING LAYOUT TO ENSURE CONTINUOUS AND SOLID BLOCKING UNDER ALL CONCENTRATED LOADS.
- CRICKETS AND/OR CALIFORNIA (BUILT-UP FRAMING):**
CRICKETS AND/OR CALIFORNIA FRAMING SHALL BE CONSTRUCTED WITH 2x6 MEMBERS AT 24" O.C. SUPPORTED TO THE ROOF BELOW AT 48 INCHES ON CENTER. ROOF PLY SHALL CONTINUE BELOW CRICKETS AND/OR CALIFORNIA FRAMING.
- HEADERS:**
WINDOW AND DOOR HEADERS SHALL CONFORM TO THE HEADER SCHEDULE, U.N.O. ON THE PLAN (210/SD2)
- TOP PLATE SPLICES:**
TOP PLATE SPLICES OF ALL WALLS SHALL CONFORM TO DETAIL (209/SD2)
INSTALL CS14 x 3'-0" LONG STRAPS AT TOP PLATE SLOPE CHANGES AND OTHER DISCONTINUOUS TOP PLATE CONDITIONS.
- TOP CONNECTORS SHALL BE INSTALLED AT EXTERIOR WALL TOP PLATES AT 4'-0" O.C. MINIMUM WHERE SHEAR IS NOT NOTED ON PLANS.**
 - SEE GENERAL NOTES FOR ADDITIONAL REQUIREMENTS. ALL DETAIL CALLOUTS SHALL BE CONSIDERED TYPICAL. THE CONTRACTOR SHALL REVIEW THE DETAIL SHEETS FOR DETAILS NOT SPECIFICALLY REFERENCED.
 - SHEARWALLS AT OPENING TRIMMERS:
WHERE SHEARWALL LENGTH SPECIFIED ON PLANS REQUIRES EDGE NAILING TO WINDOW AND/OR DOOR TRIMMERS, REFER TO DETAIL (108/SD1)
 - SHEARWALLS:**
LENGTHS OF SHEARWALLS ARE SHOWN ABOVE OR BELOW SHEARWALL NUMBER (SEE PLAN). THE NUMBERS INDICATE THE MIN. DESIGN LENGTH OF THE SHEARWALL. SEE (103/SD1)
 - HOLDOWN STRAPS**
WHERE HOLDOWN STRAPS ARE SPECIFIED ON PLANS, INSTALL PER DETAIL (112/SD1) (113/SD1)
 - ALL NAIL HOLES SHALL BE FILLED ON HOLDOWN STRAPS**



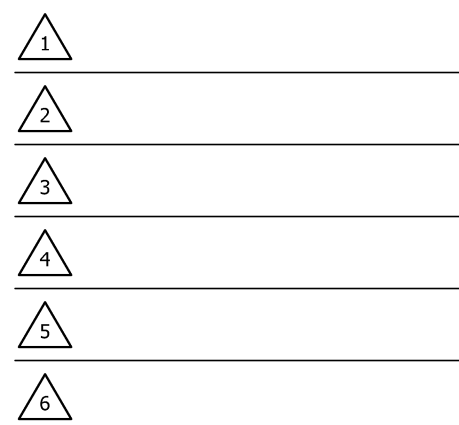
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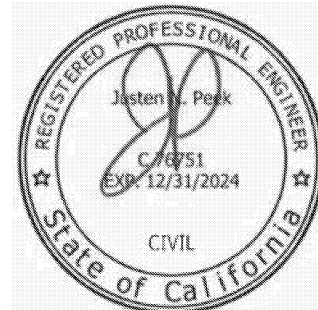
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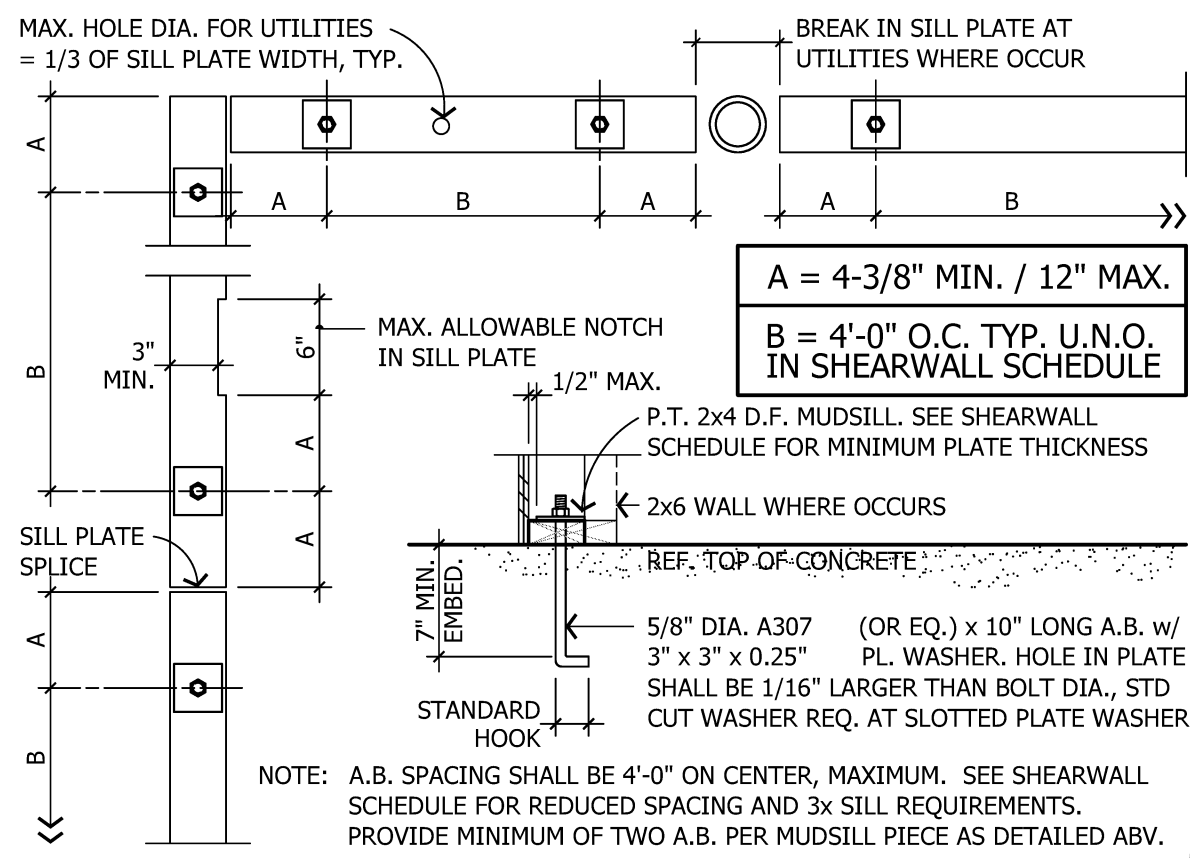


Sheet Description:
**1 BEDROOM UNIT
STRUCTURAL ROOF FRAMING
PLAN AND FOUNDATION PLAN**

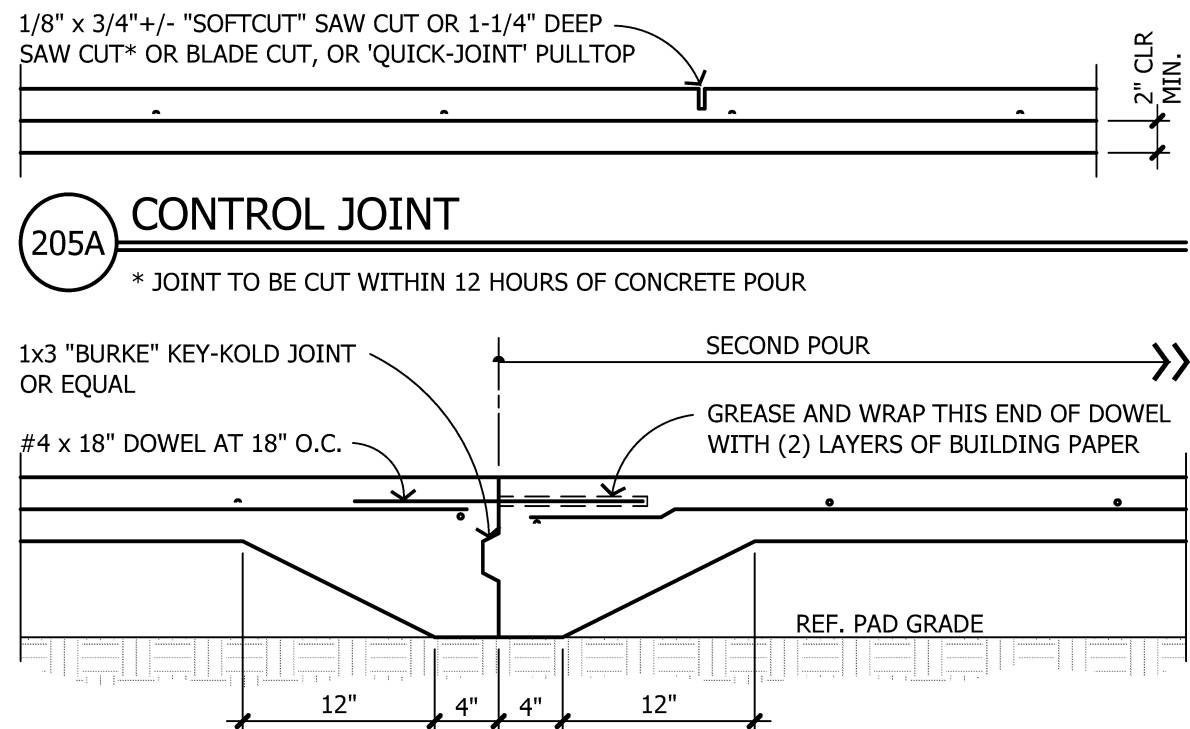
Scale: 1/4" = 1'-0"
Initial Issue Date: March 24, 2023
Drawn By: E. Bennett
Project Designer: J. Peek
Project Manager: J. Peek Ext. 23
Job No. W021420

Sheet No.

S1

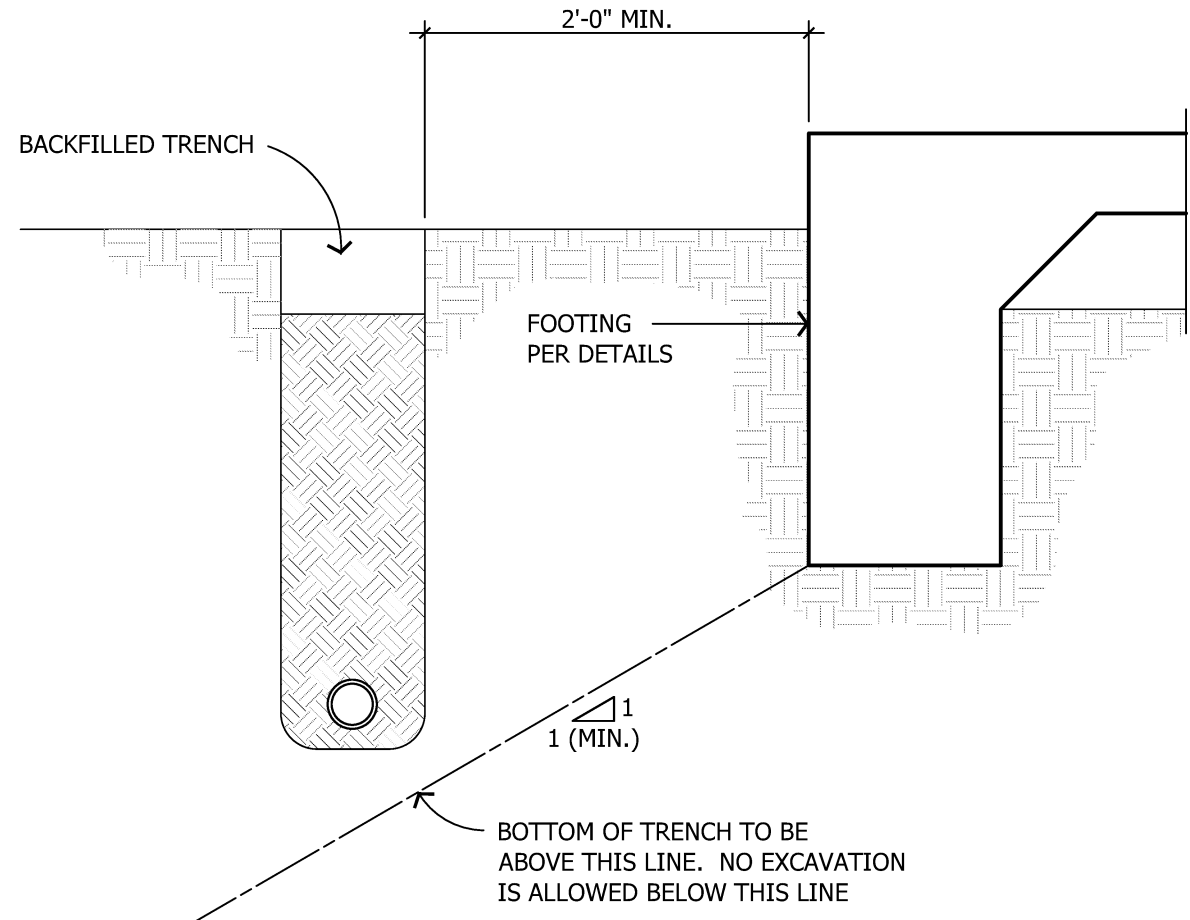


201 TYPICAL ANCHOR BOLT AND SHOT PIN PLACEMENT

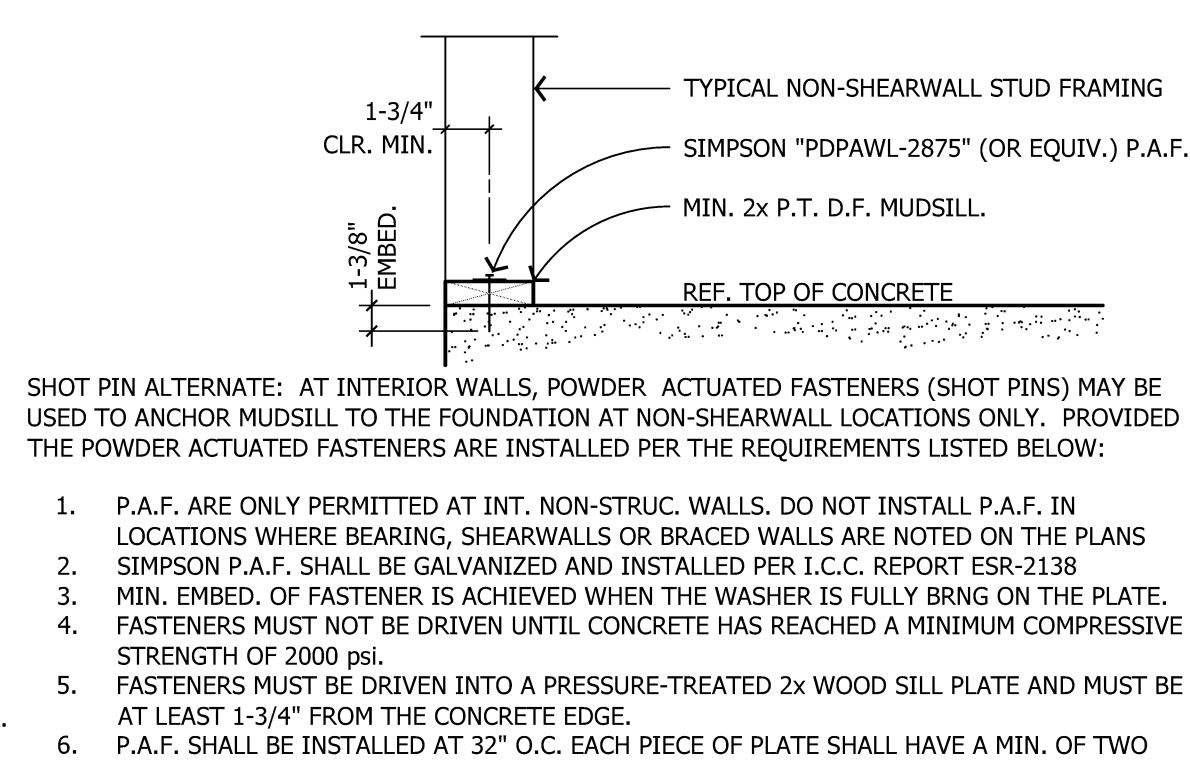


205B CONSTRUCTION JOINT

205 CRACK CONTROL JOINTS (SLAB-ON-GRADE)

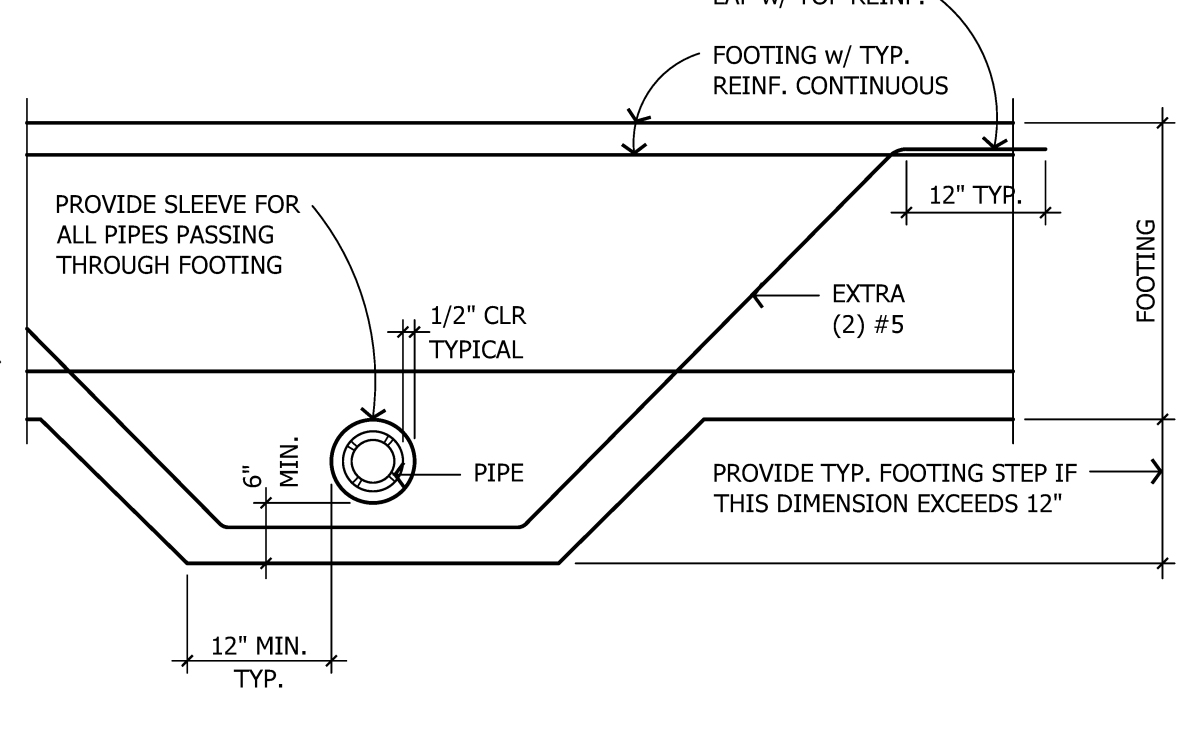


207 PIPE / TRENCH PARALLEL TO FOOTING



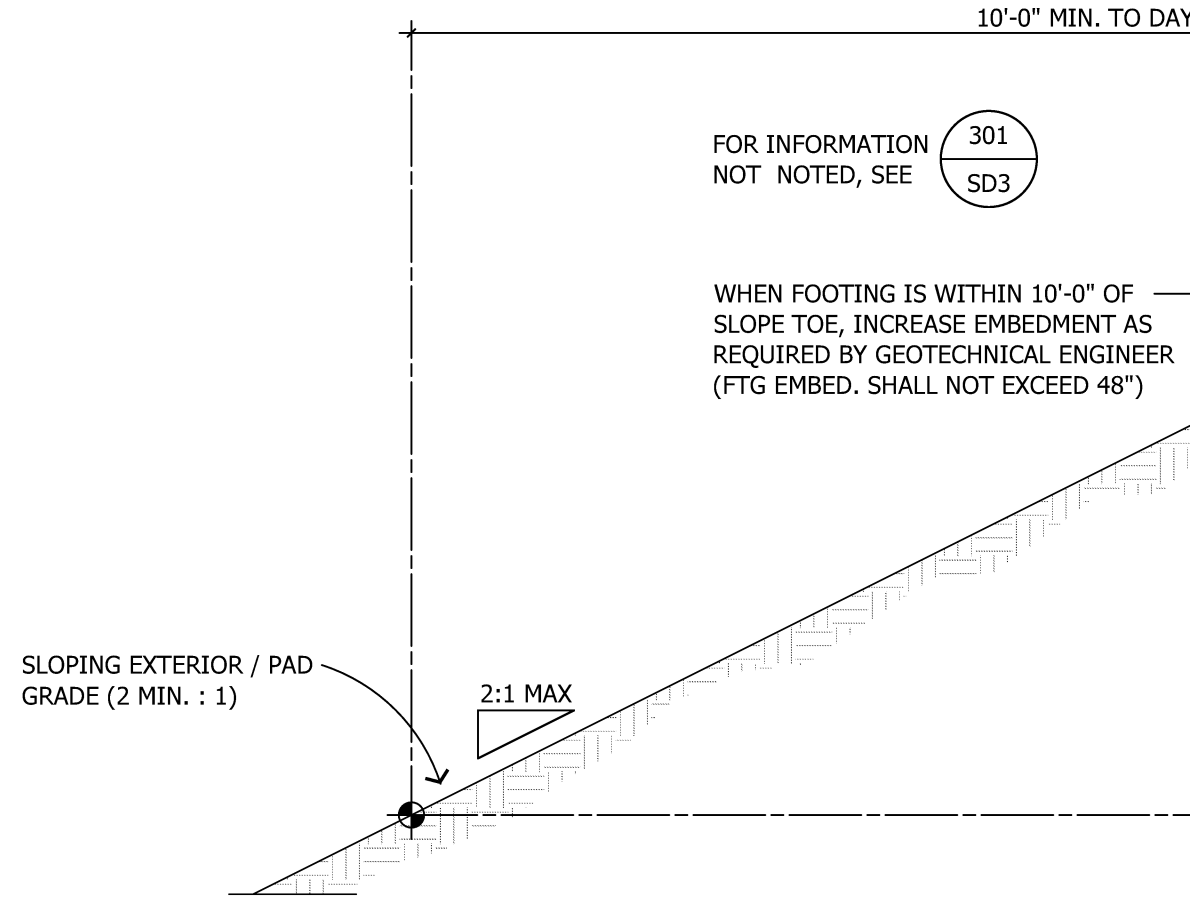
201A SHOT PIN PLACEMENT AT NON-STRUCTURAL WALLS

202 TYP. REINFORCING HOOKS AND LAPS

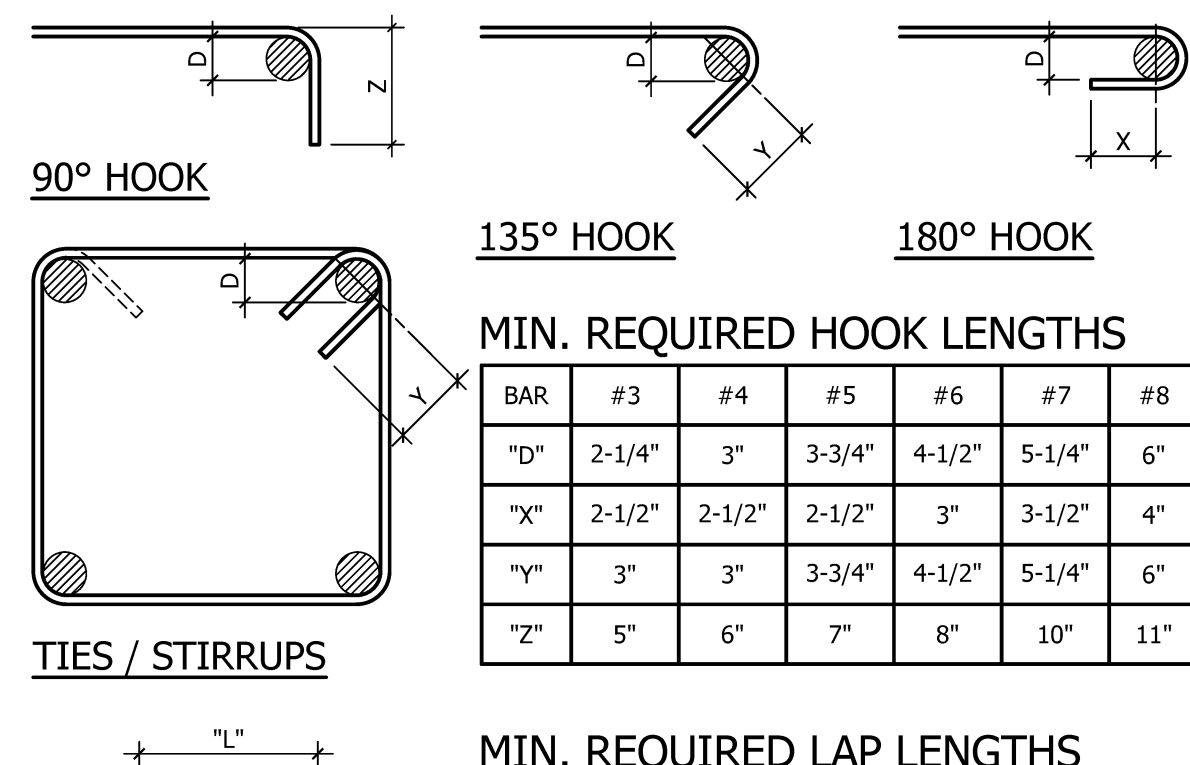


206A PIPE BELOW FOOTING

206 PIPE PERPENDICULAR TO FOOTING

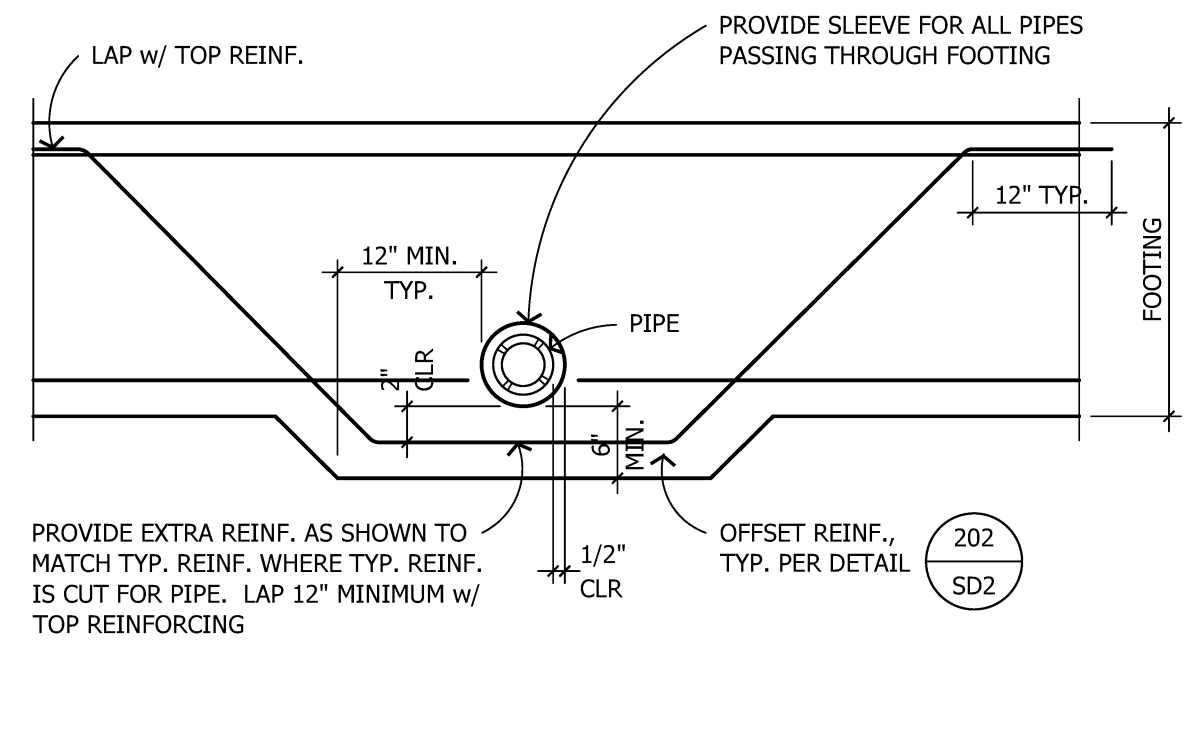


208 INCREASED FOOTING DEPTH BY SLOPE



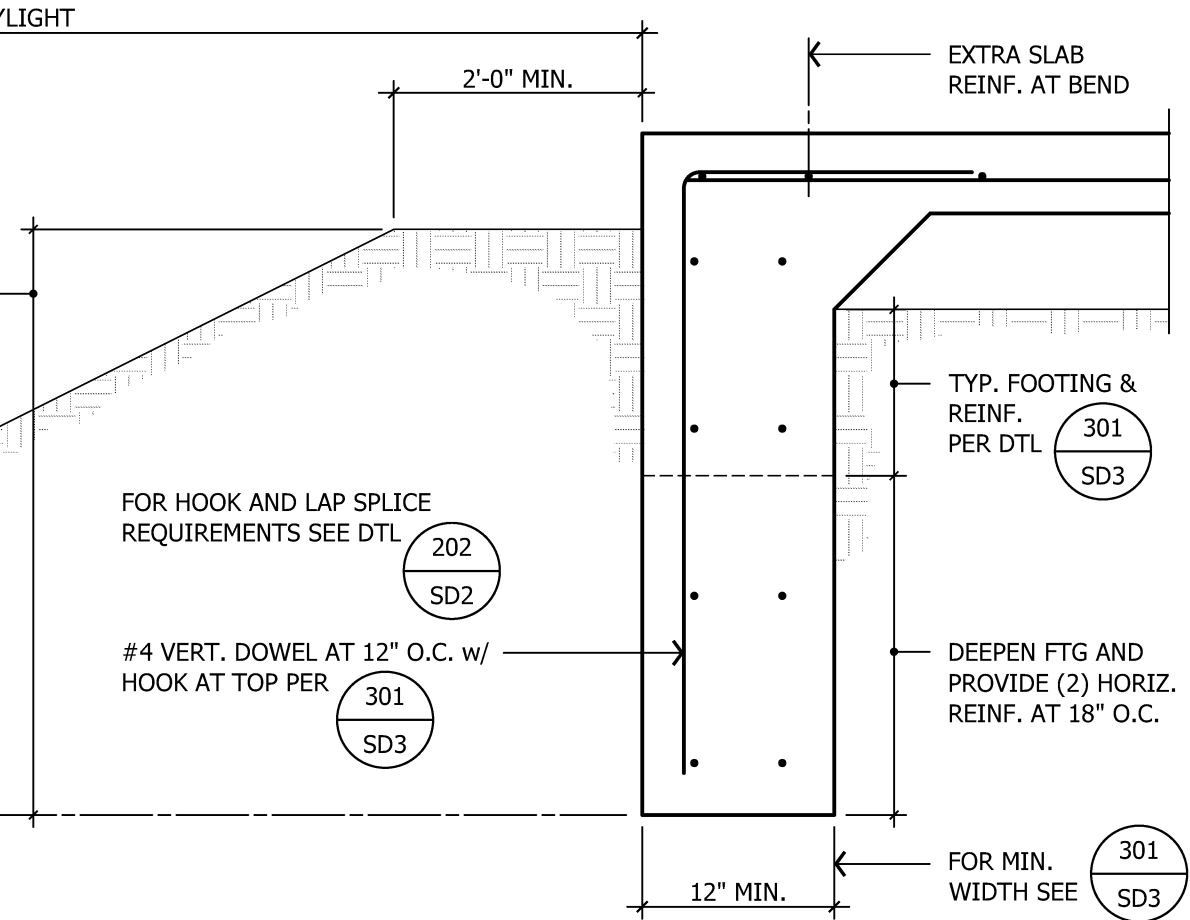
203A TEE COND.

203B CORNER COND.

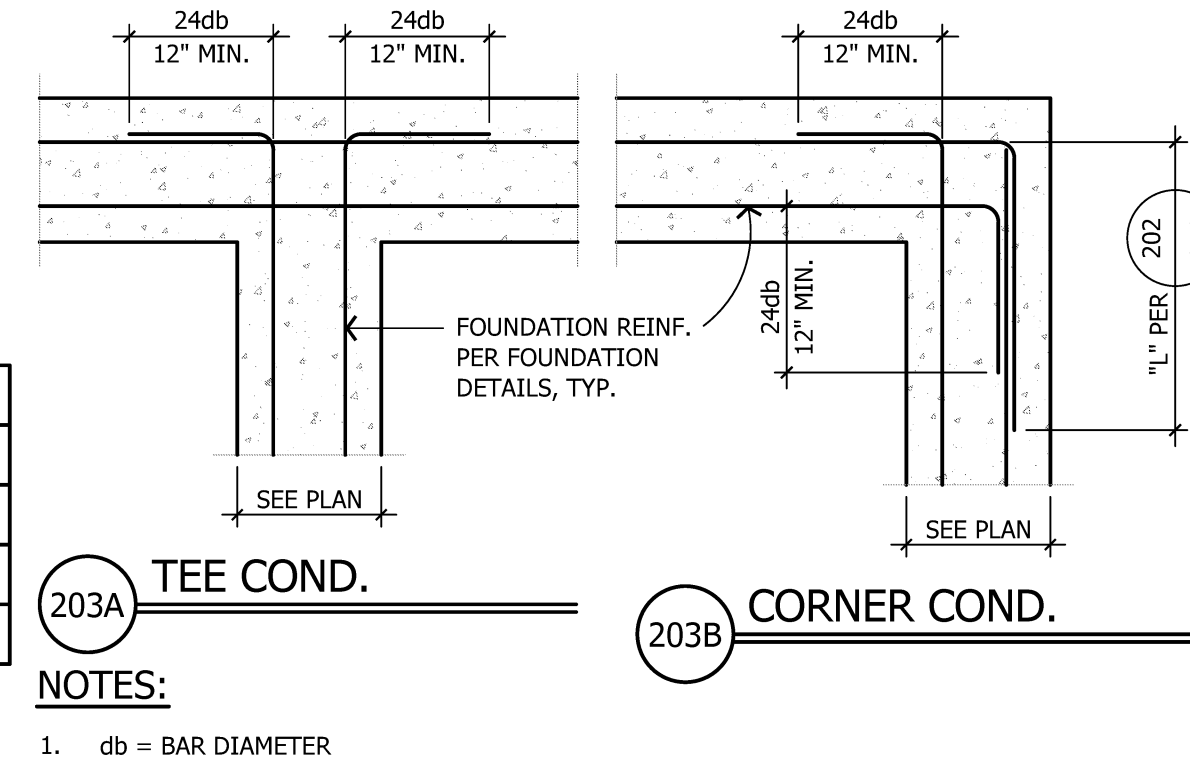


206B PIPE THROUGH FOOTING

206C PIPE BELOW FOOTING

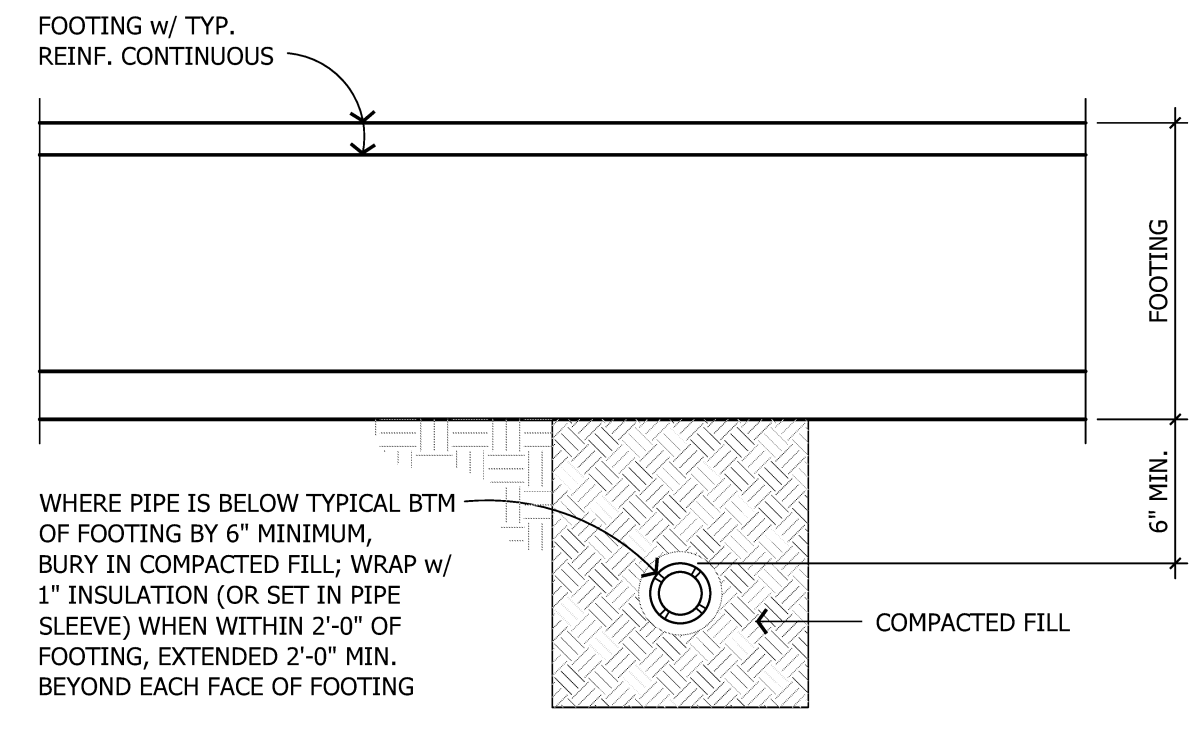


209A TYPICAL LAP SPLICE



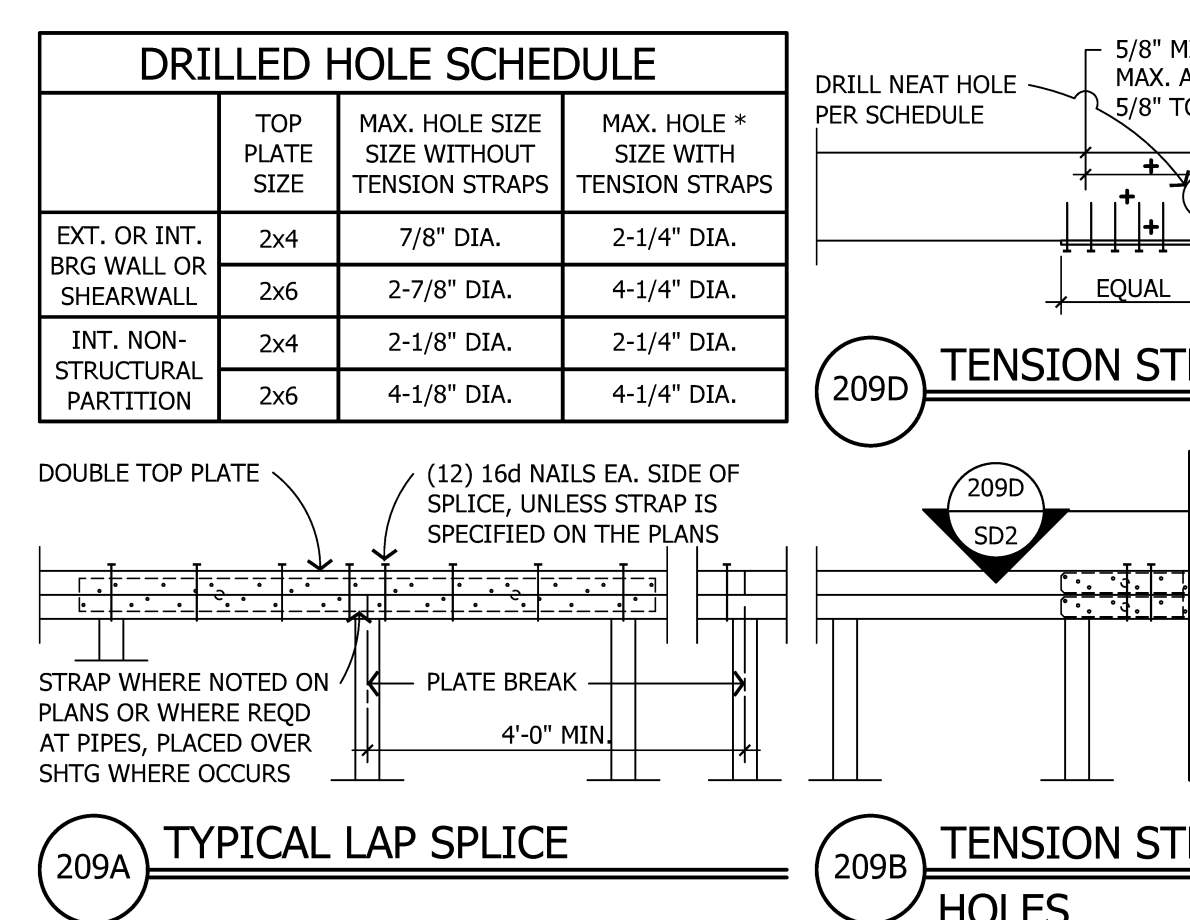
209B TENSION STRAP AT DRILLED HOLES

209C COMPRESSION STRAP AT NOTCHES

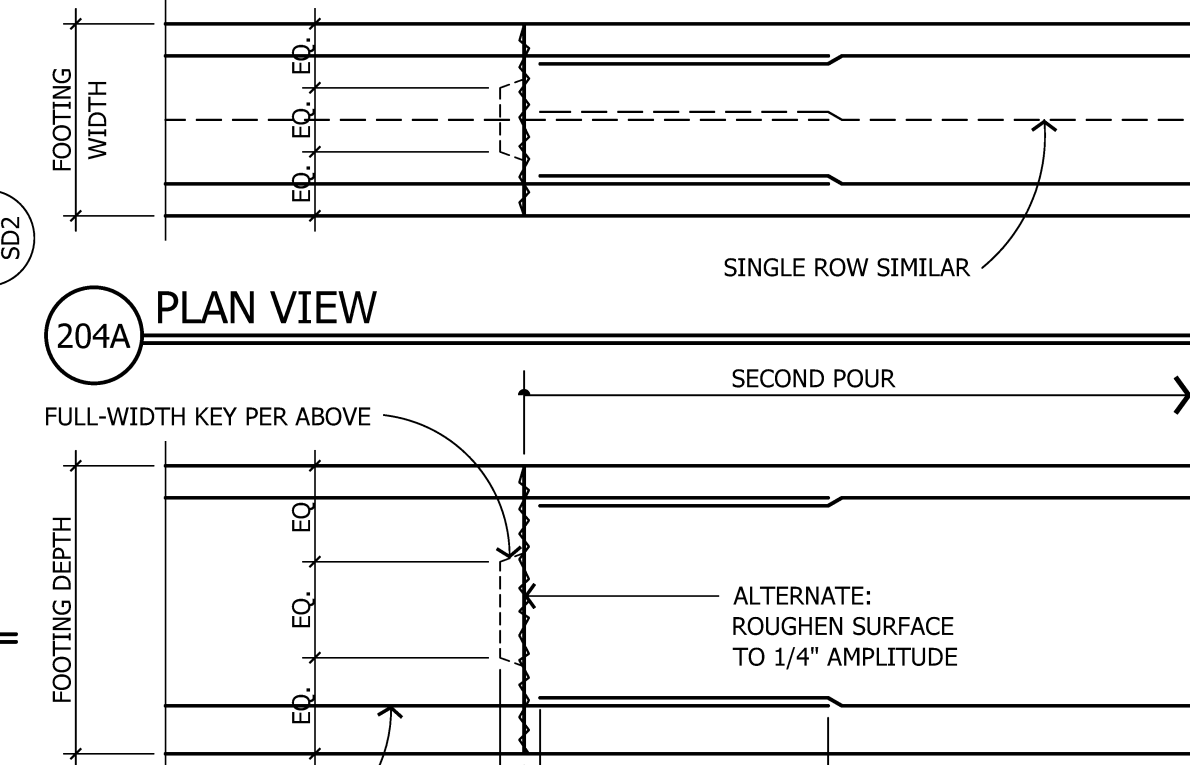


209C COMPRESSION STRAP AT NOTCHES

209D TENSION STRAP - PLAN VIEW

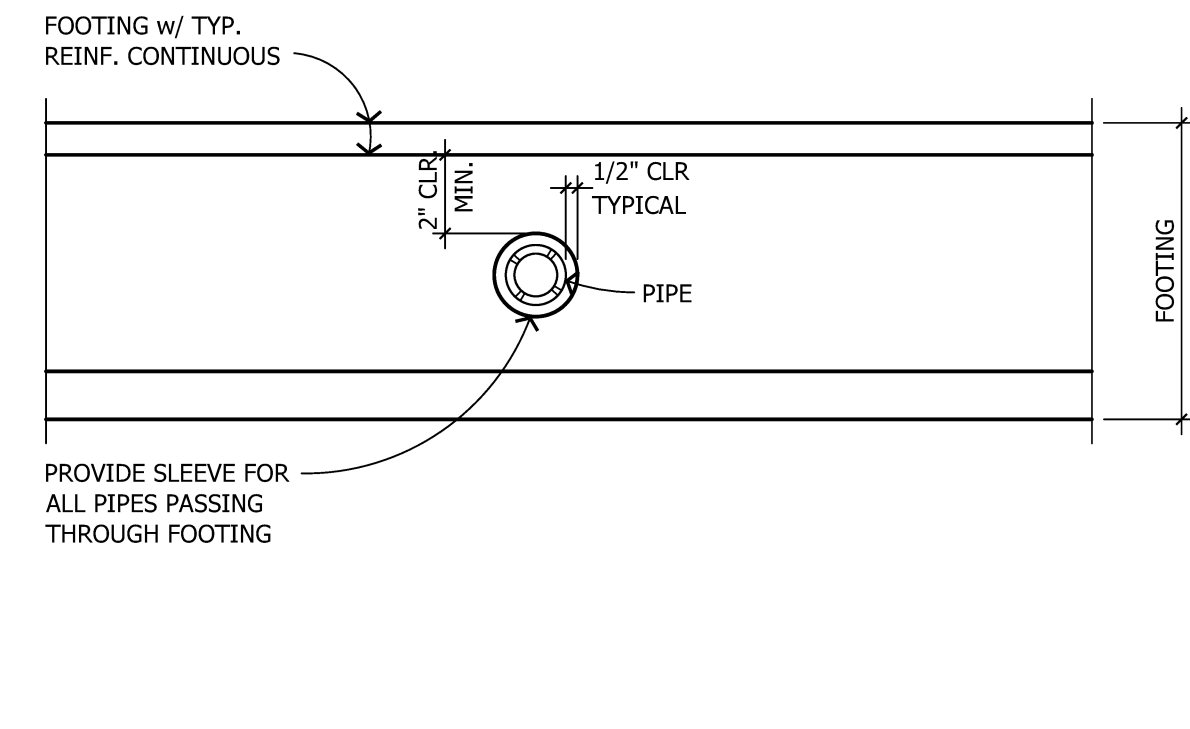


209E COMPRESSION STRAP - PLAN VIEW



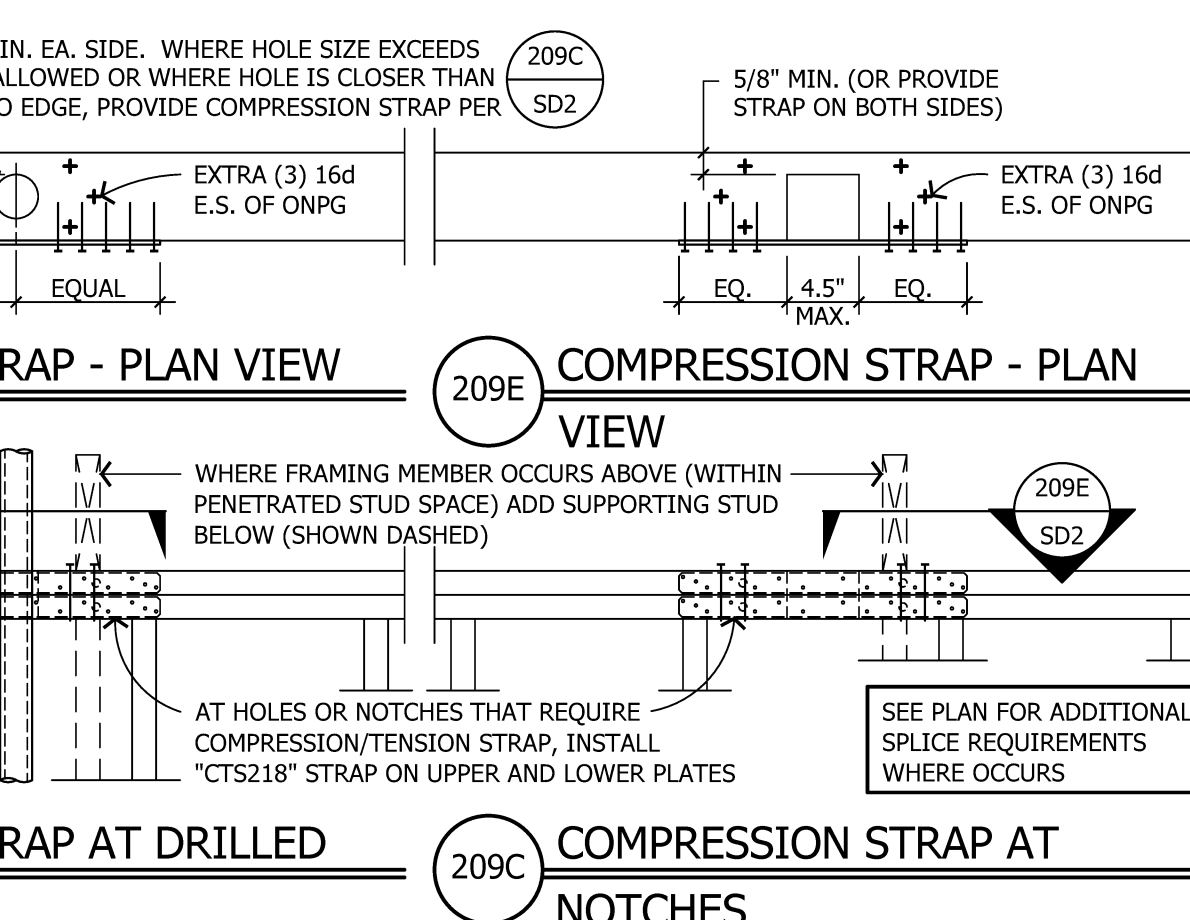
204 FOOTING CONSTRUCTION JOINT

206C PIPE BELOW FOOTING

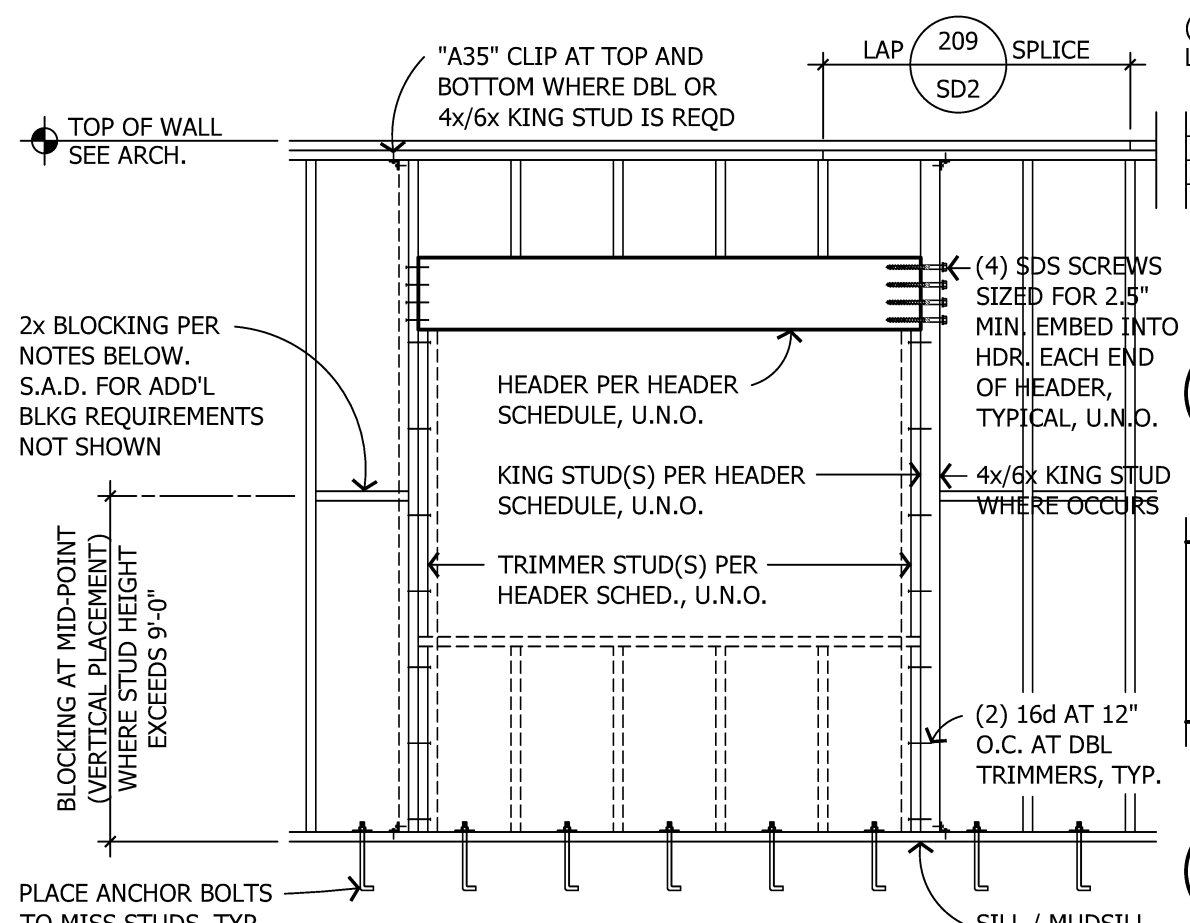


206D PIPE BETWEEN BARS

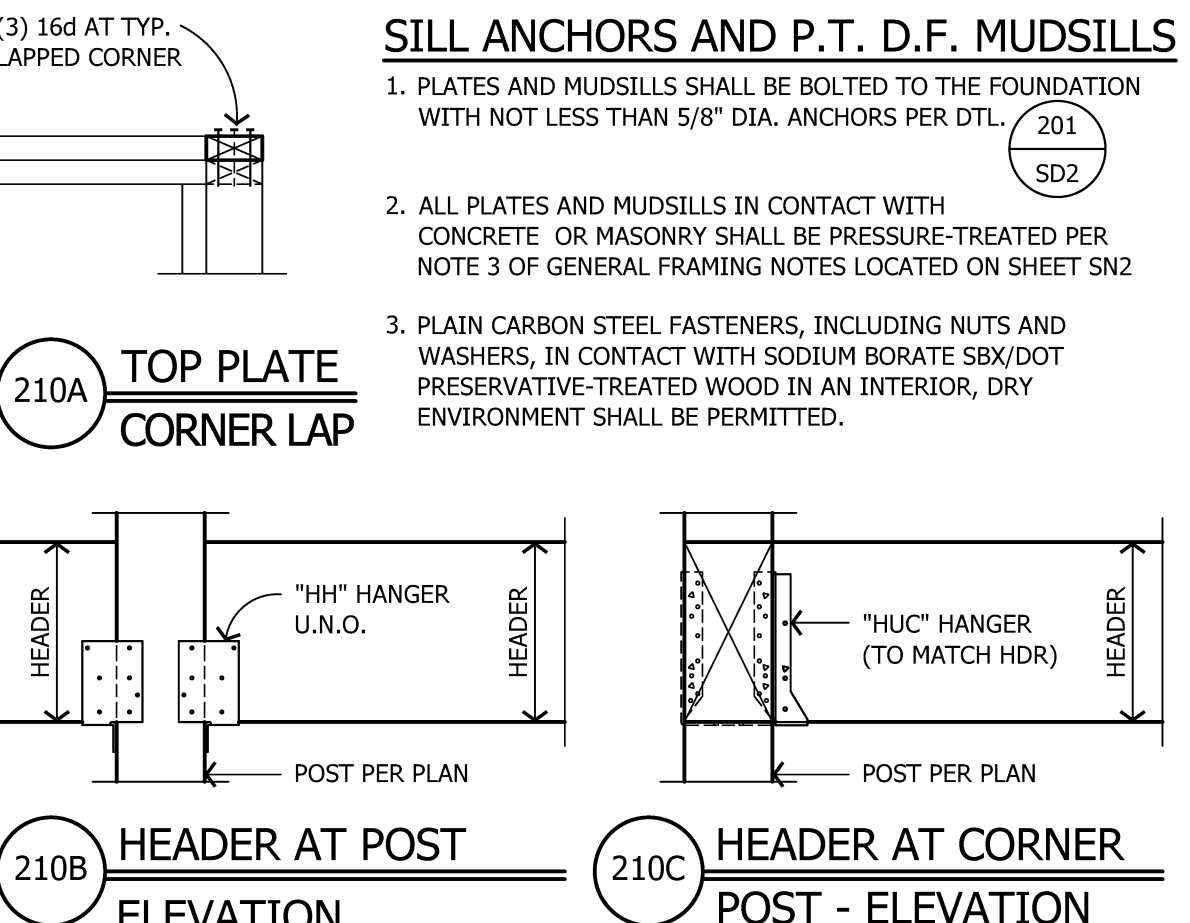
209D TENSION STRAP - PLAN VIEW



209E COMPRESSION STRAP - PLAN VIEW

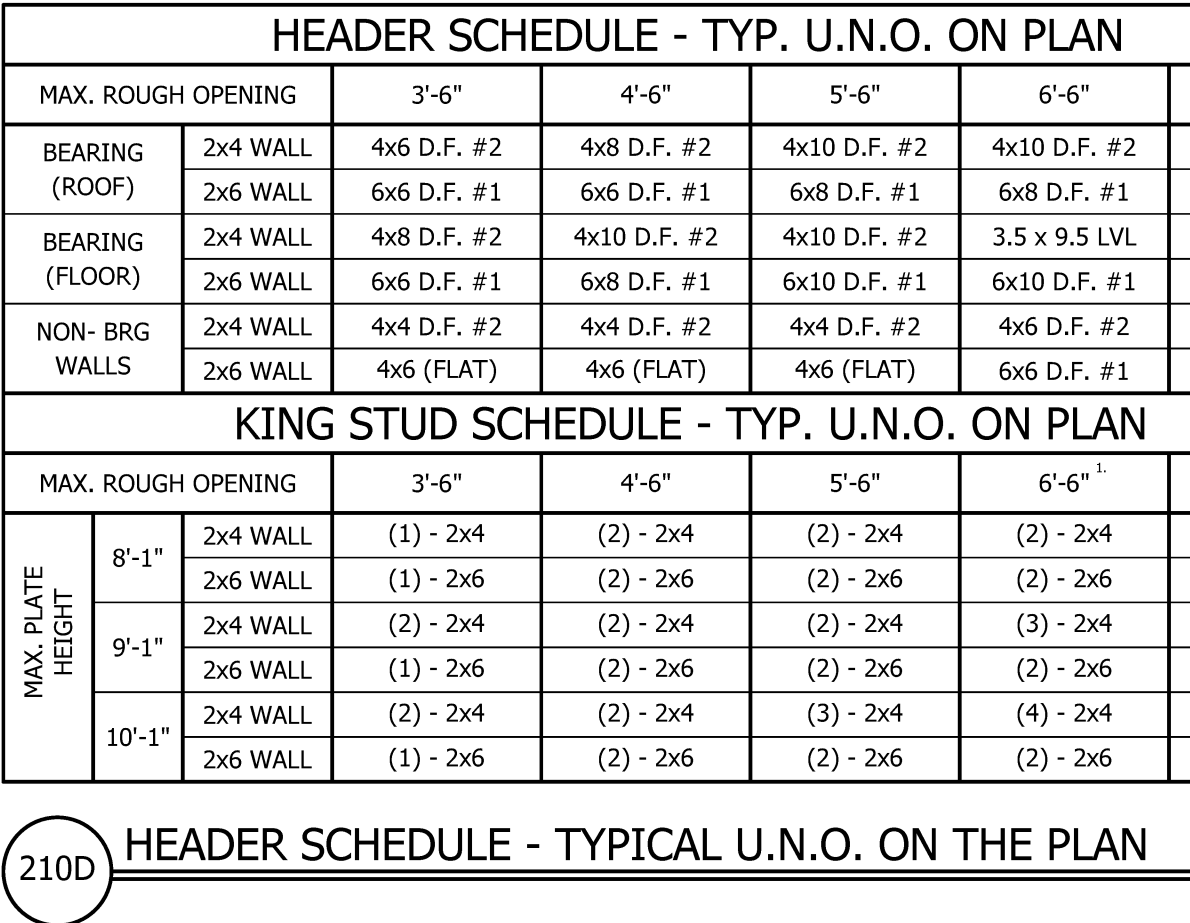


210 TYPICAL FRAMED WALL OPENING AND HEADER SCHEDULE



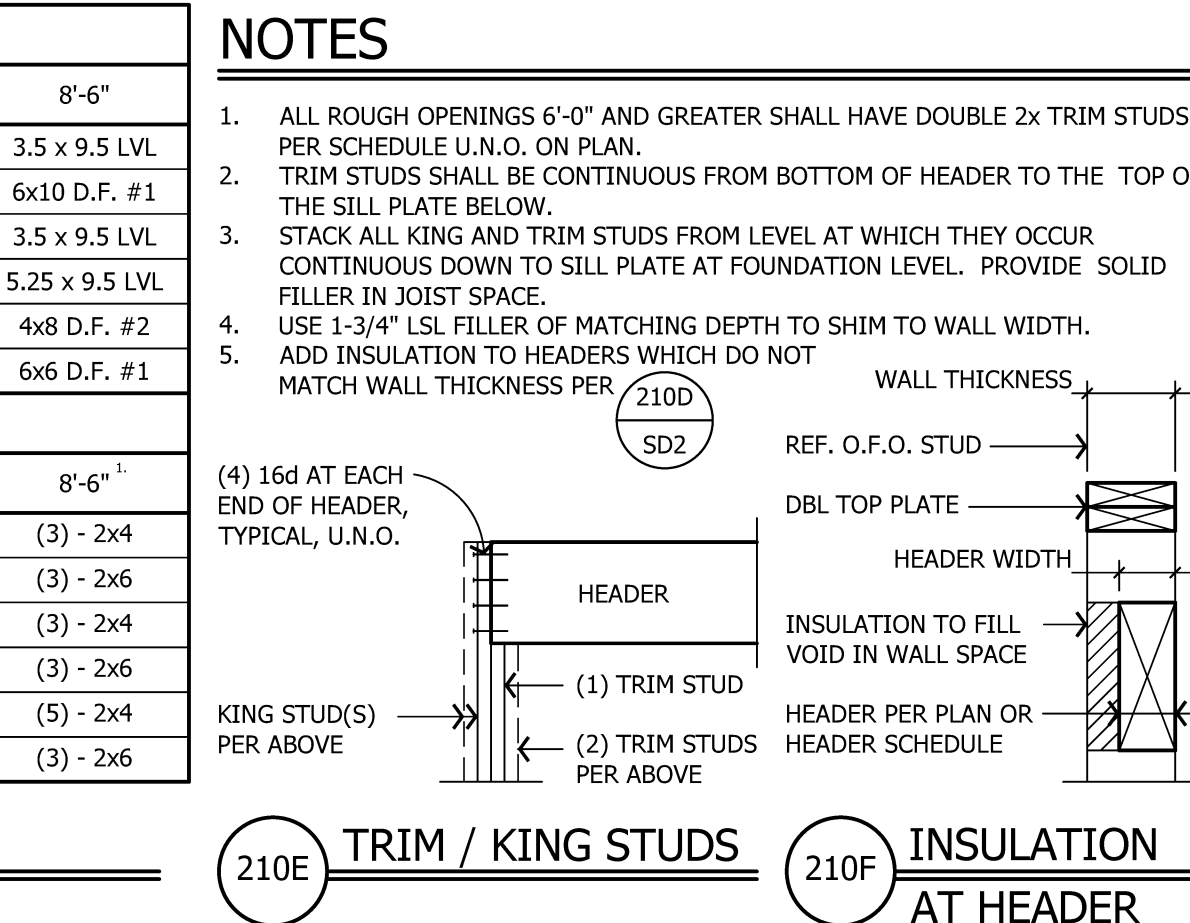
210A TOP PLATE CORNER LAP

210B HEADER AT POST ELEVATION



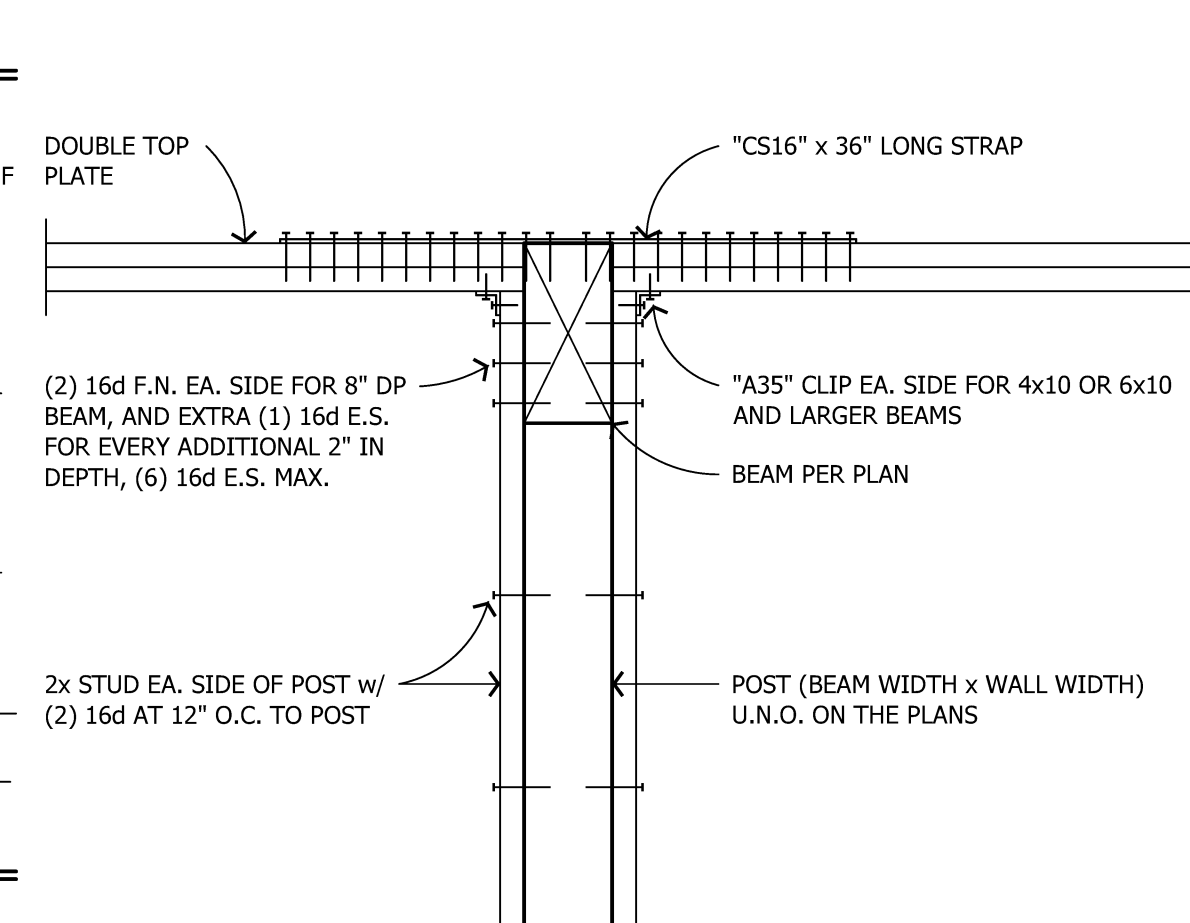
210C HEADER AT CORNER POST - ELEVATION

210D HEADER SCHEDULE - TYPICAL U.N.O. ON THE PLAN

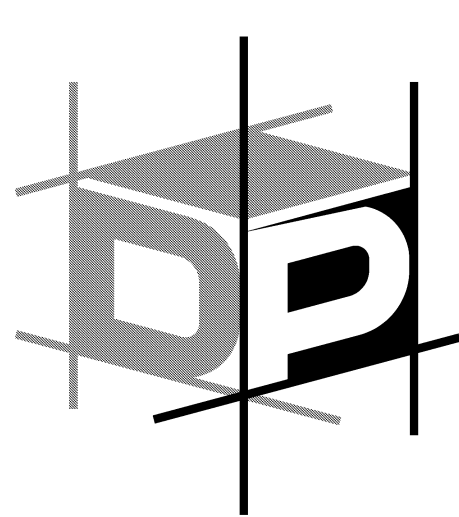


210E TRIM / KING STUDS

210F INSULATION AT HEADER



211 TYPICAL BEAM SUPPORT IN WALL



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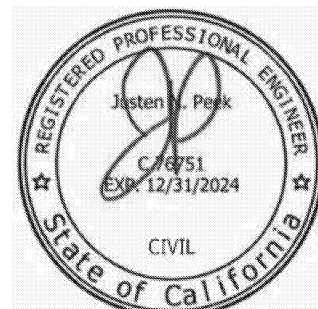
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TYPICAL CONCRETE AND CARPENTRY DETAILS

No Scale

Initial Issue Date: March 24, 2023

Drawn By: E. Bennett

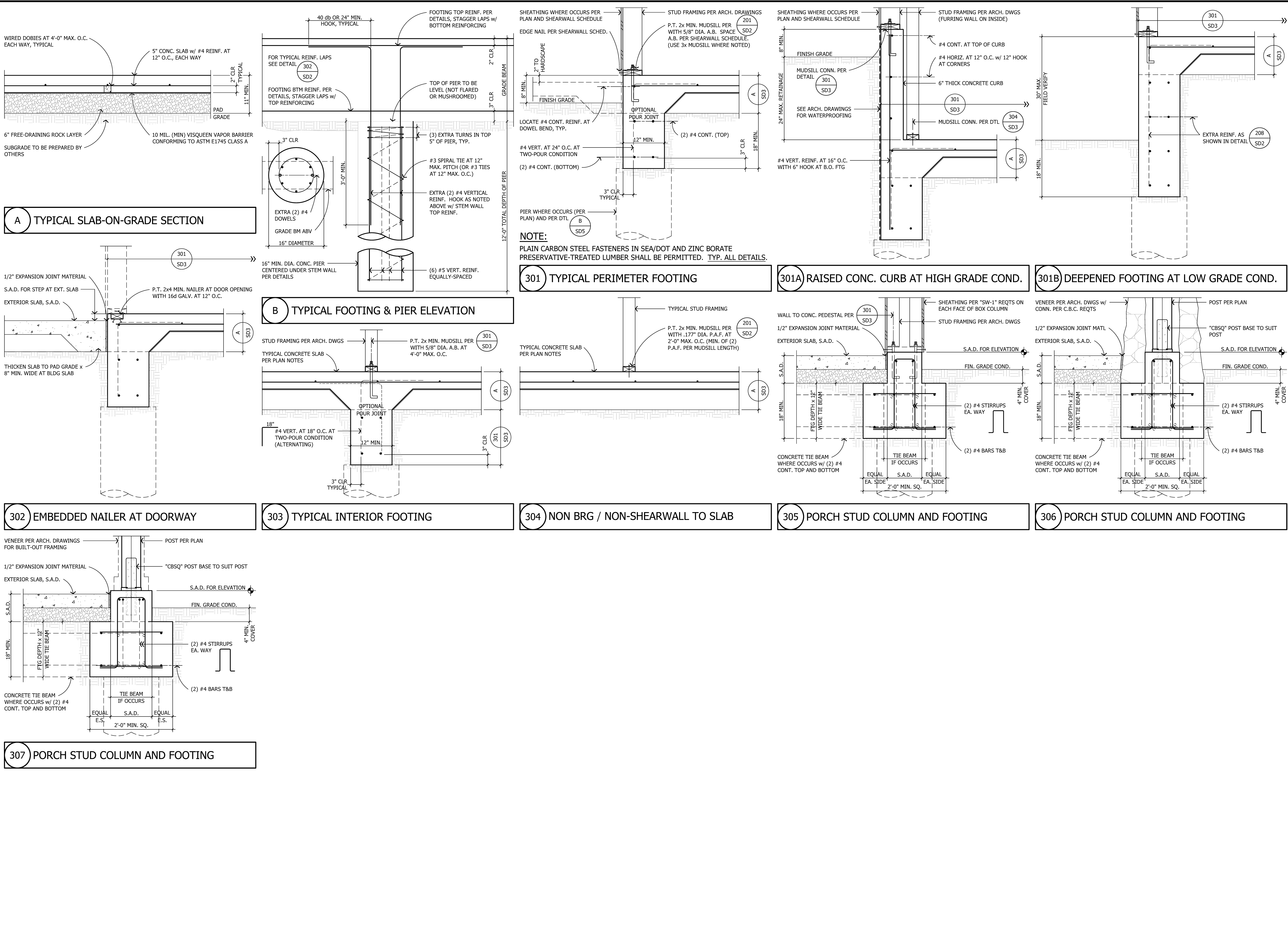
Project Designer: J. Peek

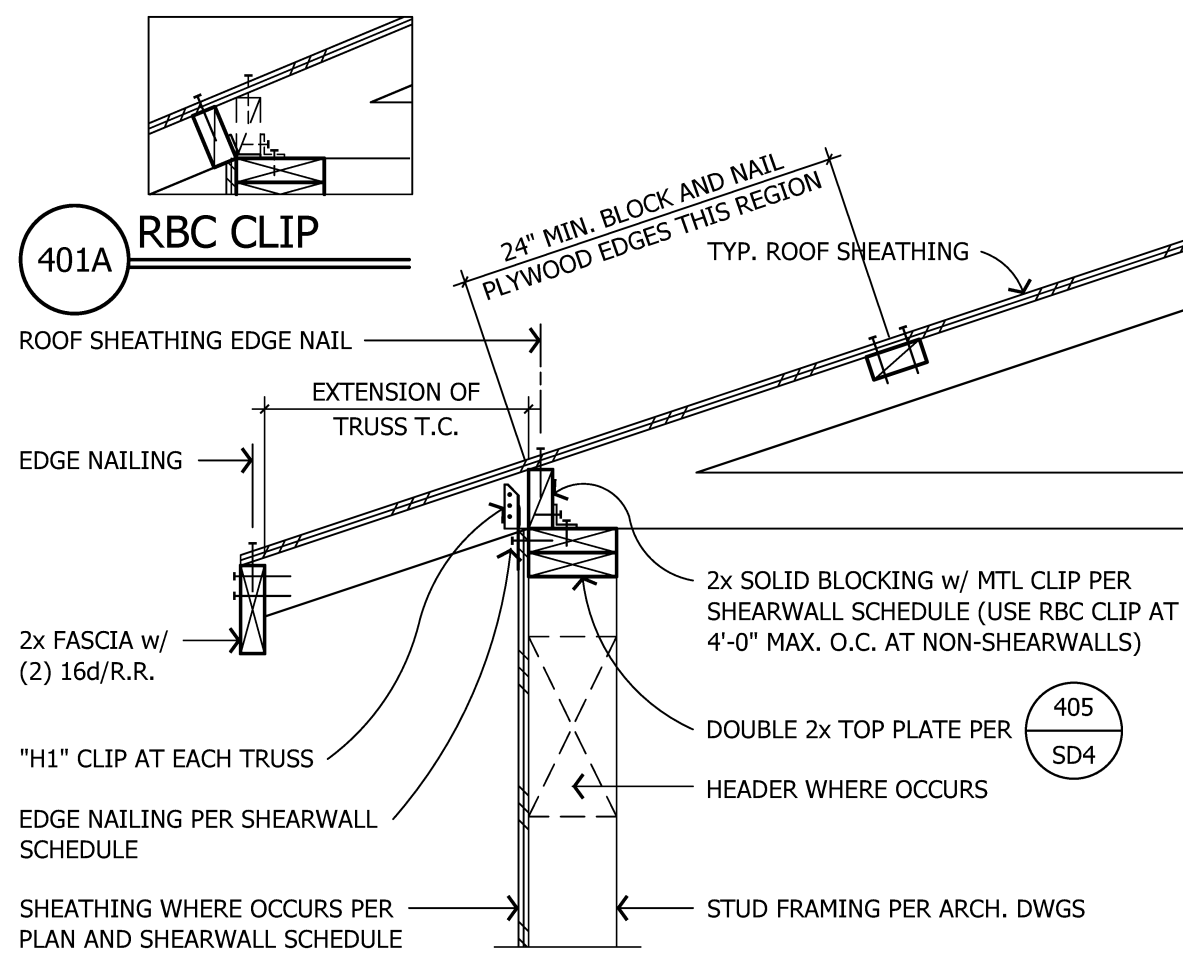
Project Manager: J. Peek Ext. 23

Job No. W021420

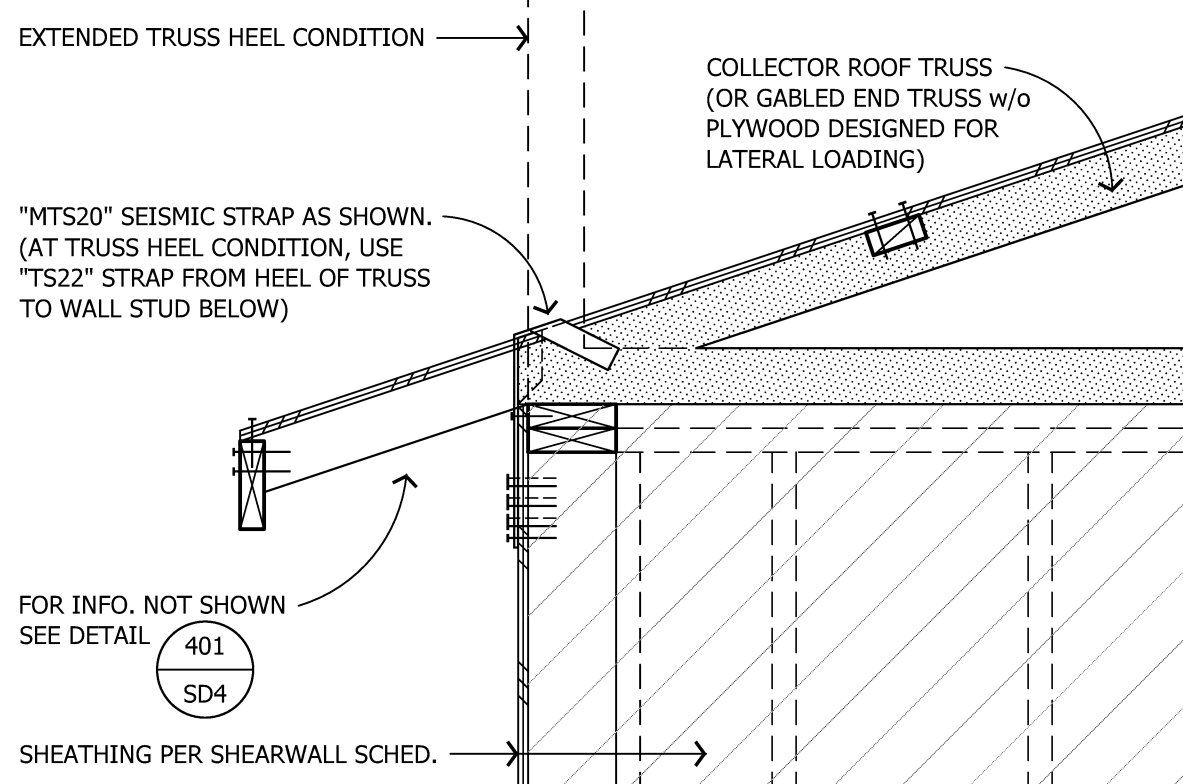
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SD2

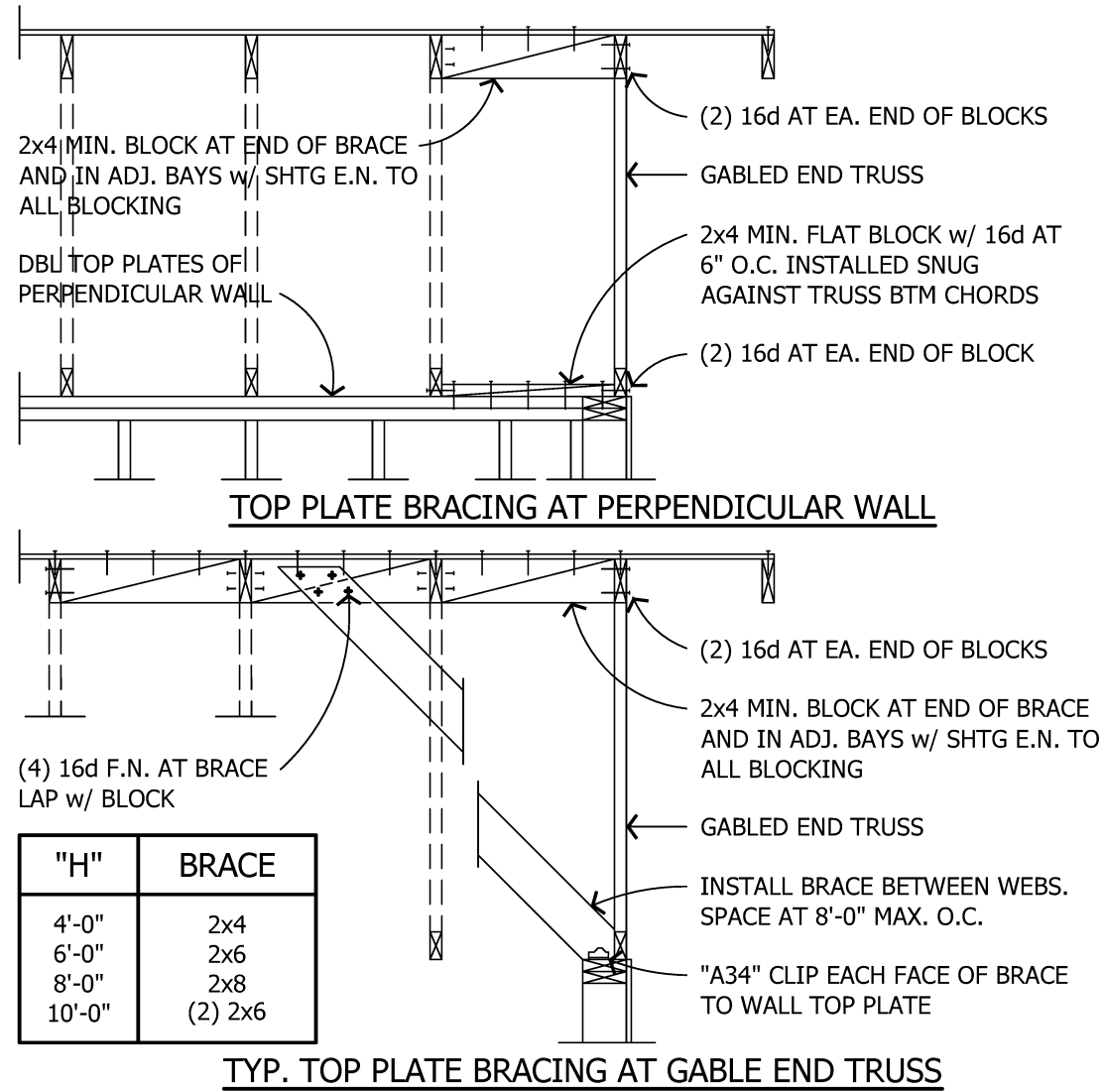




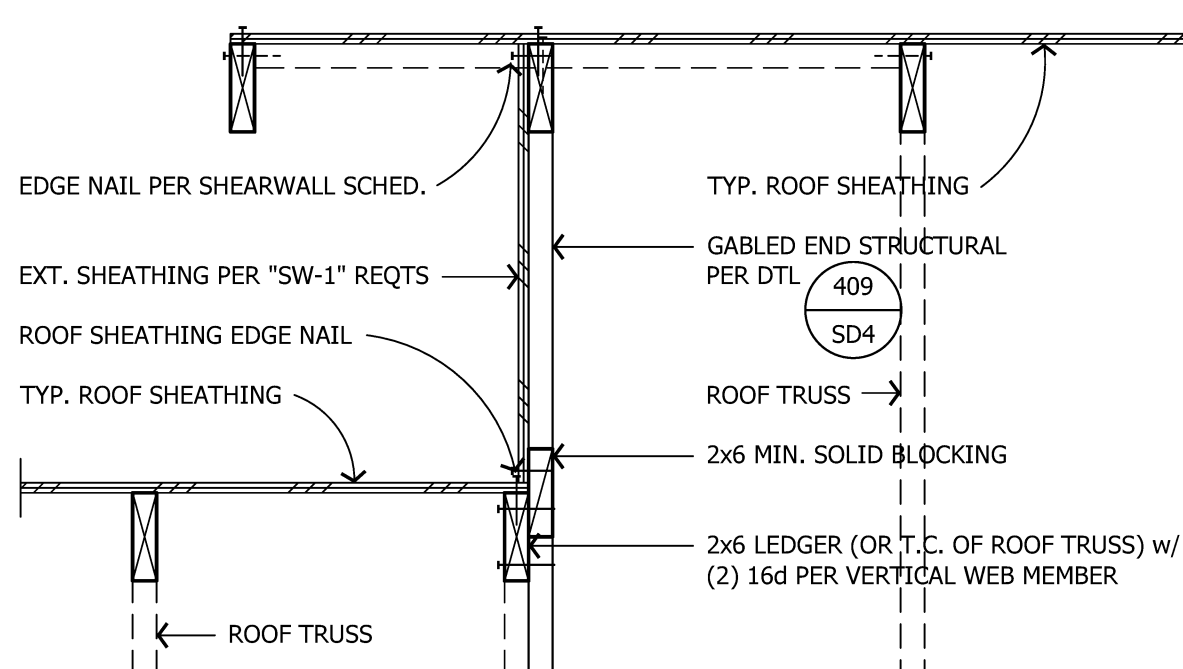
401 TRUSS PERPEND. TO EXTERIOR WALL



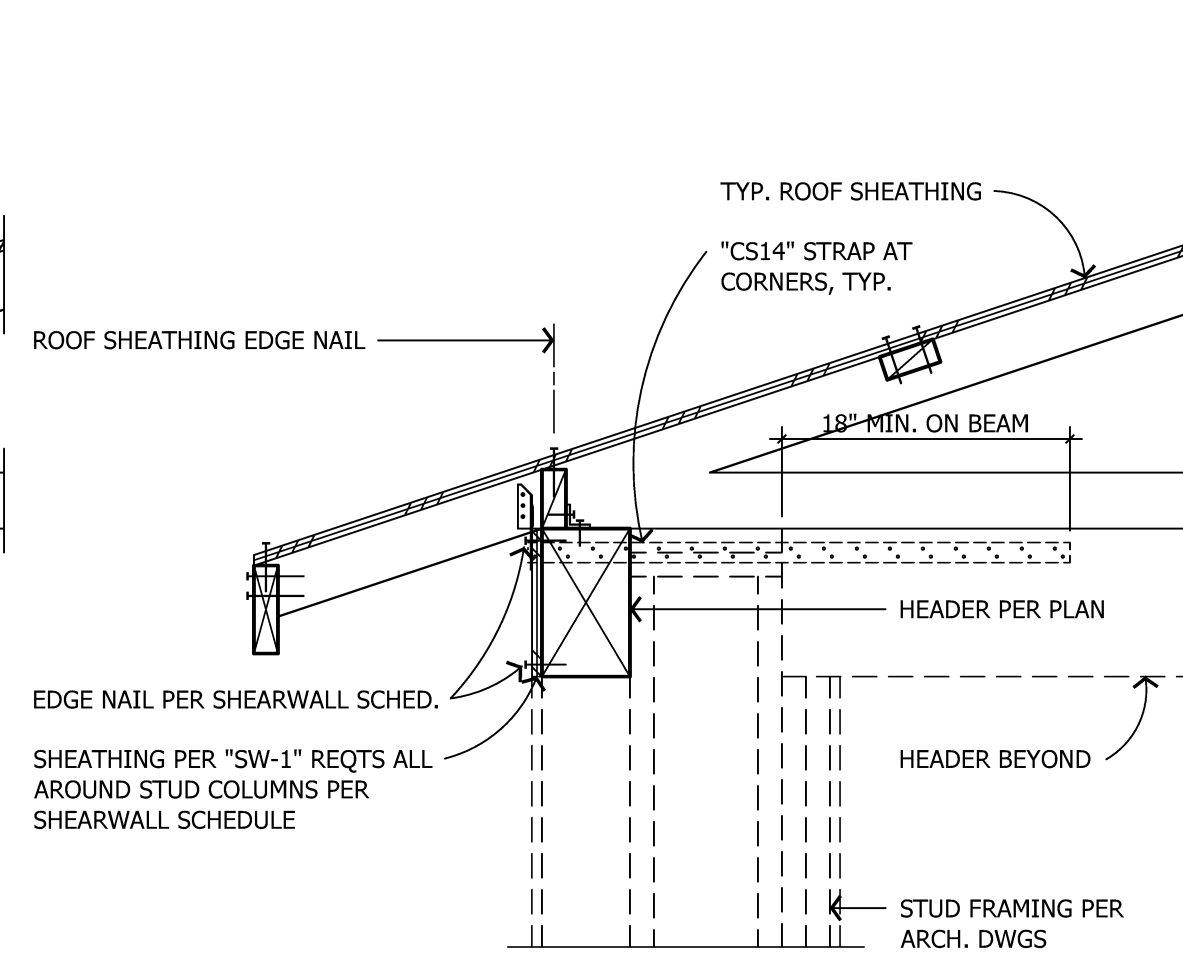
406 COLLECTOR TRUSS UPLIFT ANCHORAGE



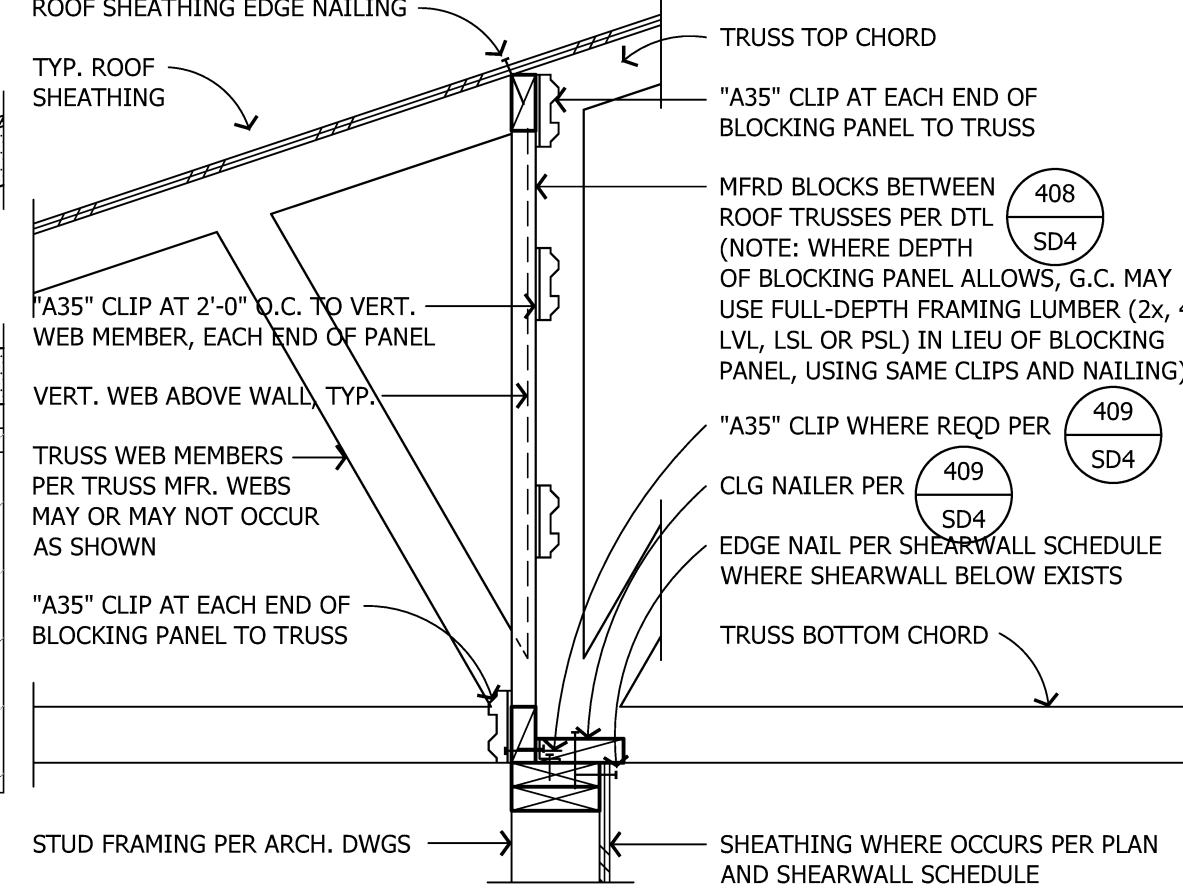
410 GABLE END TRUSS BRACING



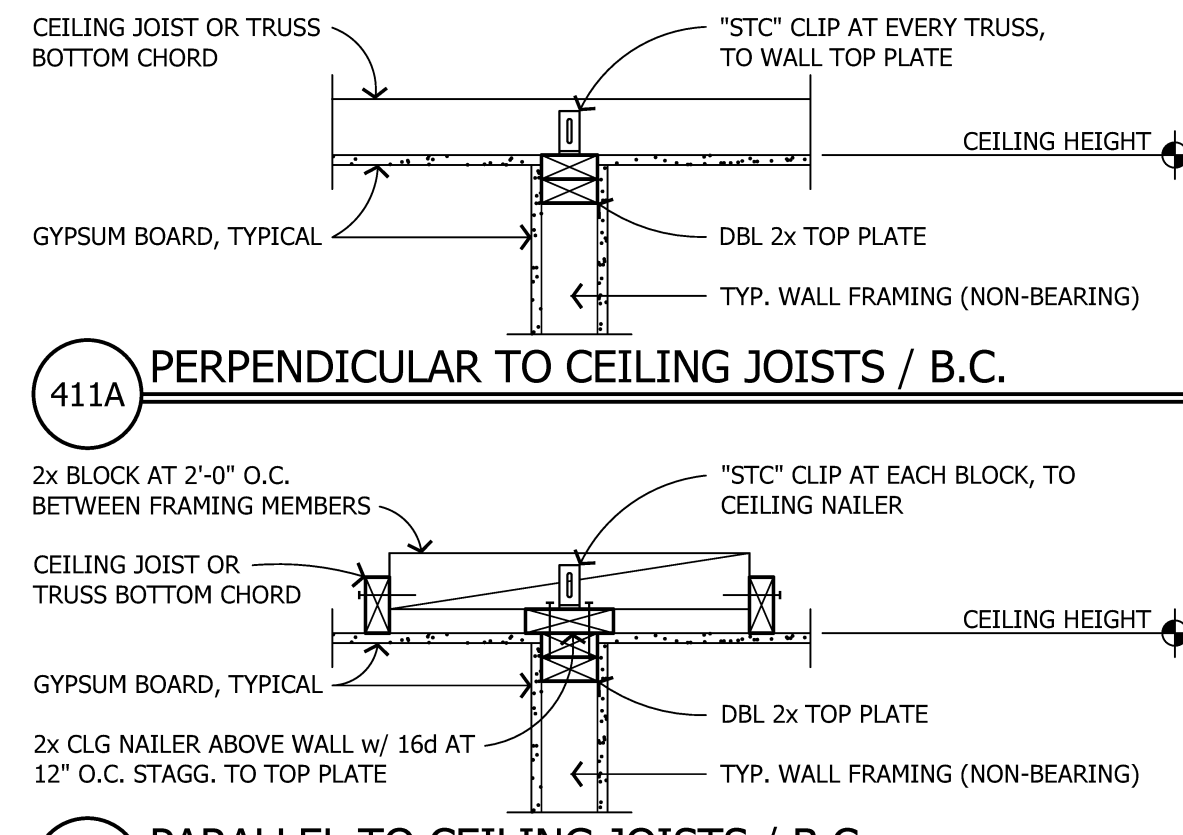
415 ROOF STEP



402 TRUSS PERPEND. TO PORCH BEAM

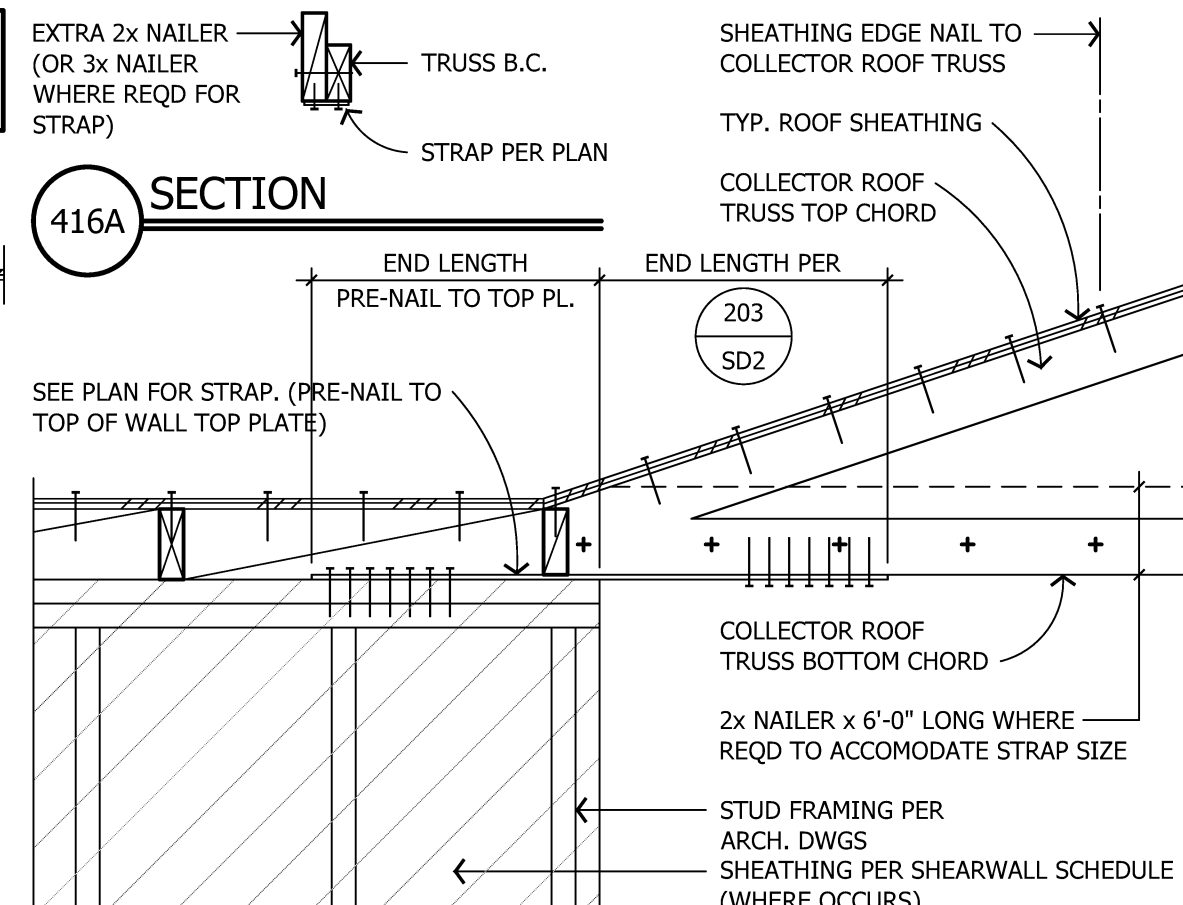


407 TRUSS TO INTERIOR SHEAR WALL

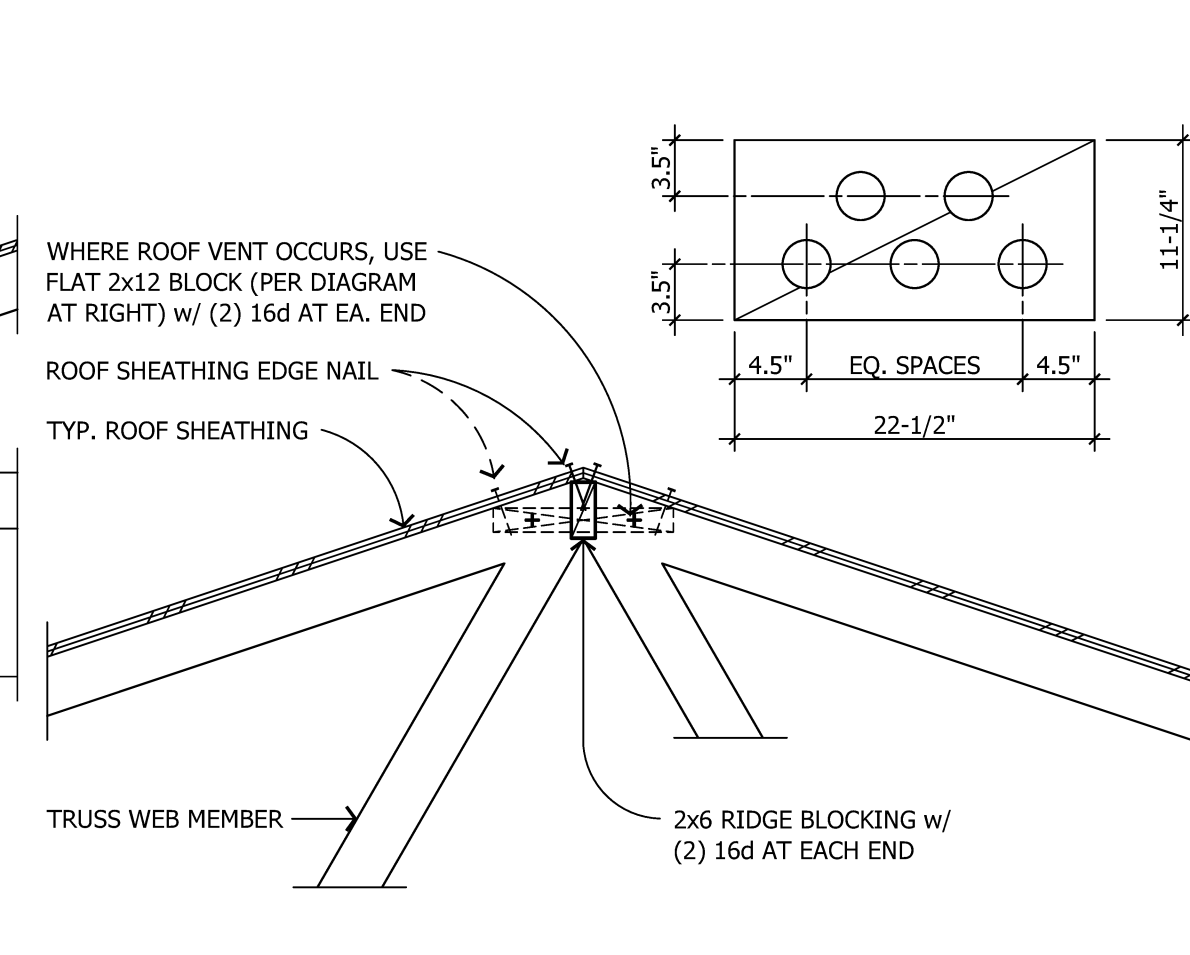


411B PARALLEL TO CEILING JOISTS / B.C.

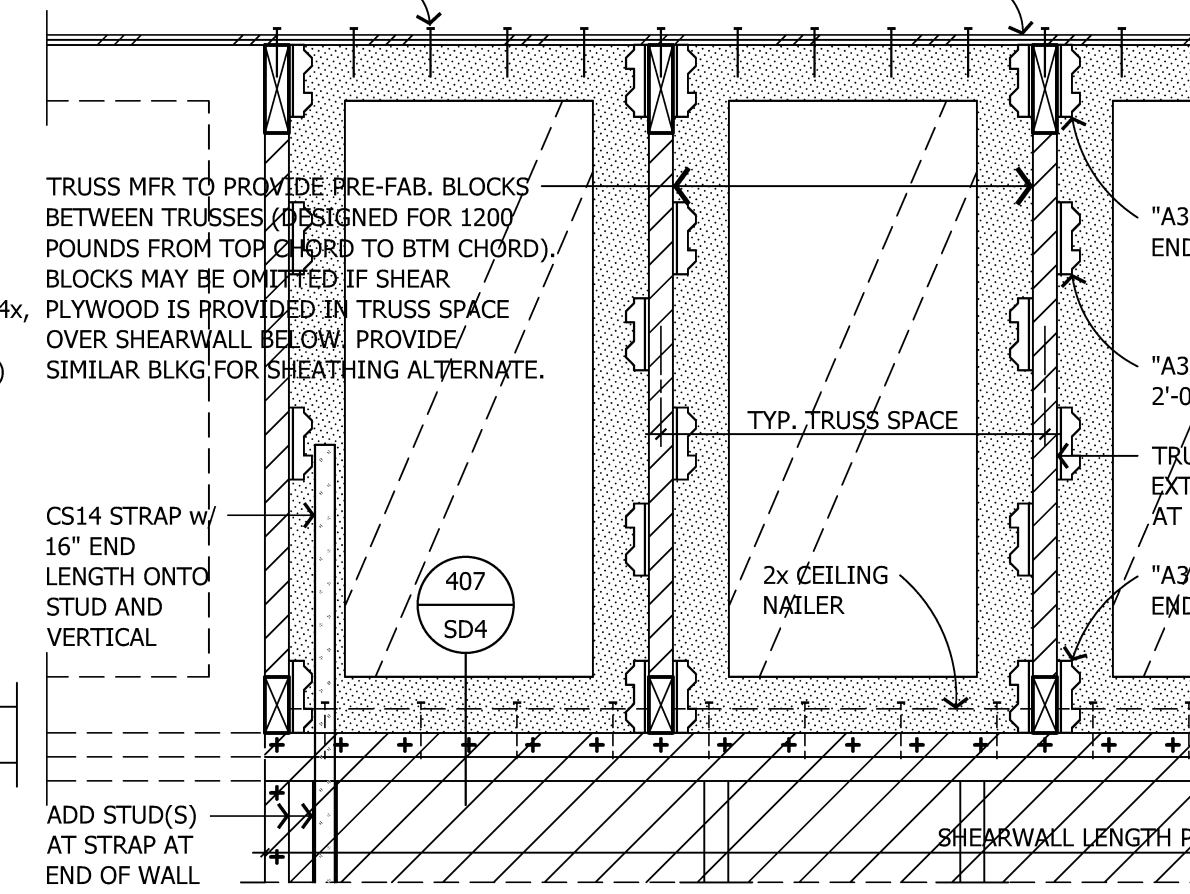
411 INTERIOR NON-BEARING WALL TO TRUSS



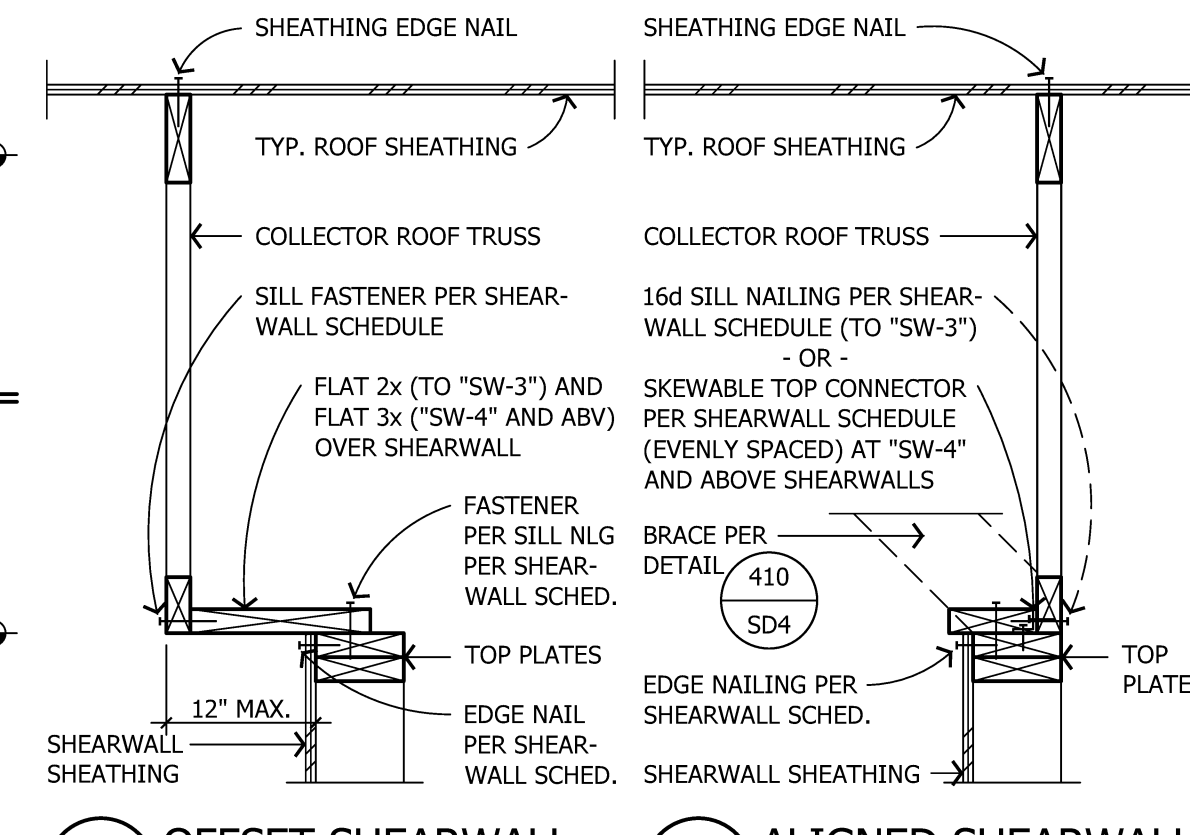
416 HORIZONTAL STRAP TO WALL TOP PLATE



403 RIDGE BLOCKING AT ROOF TRUSSES

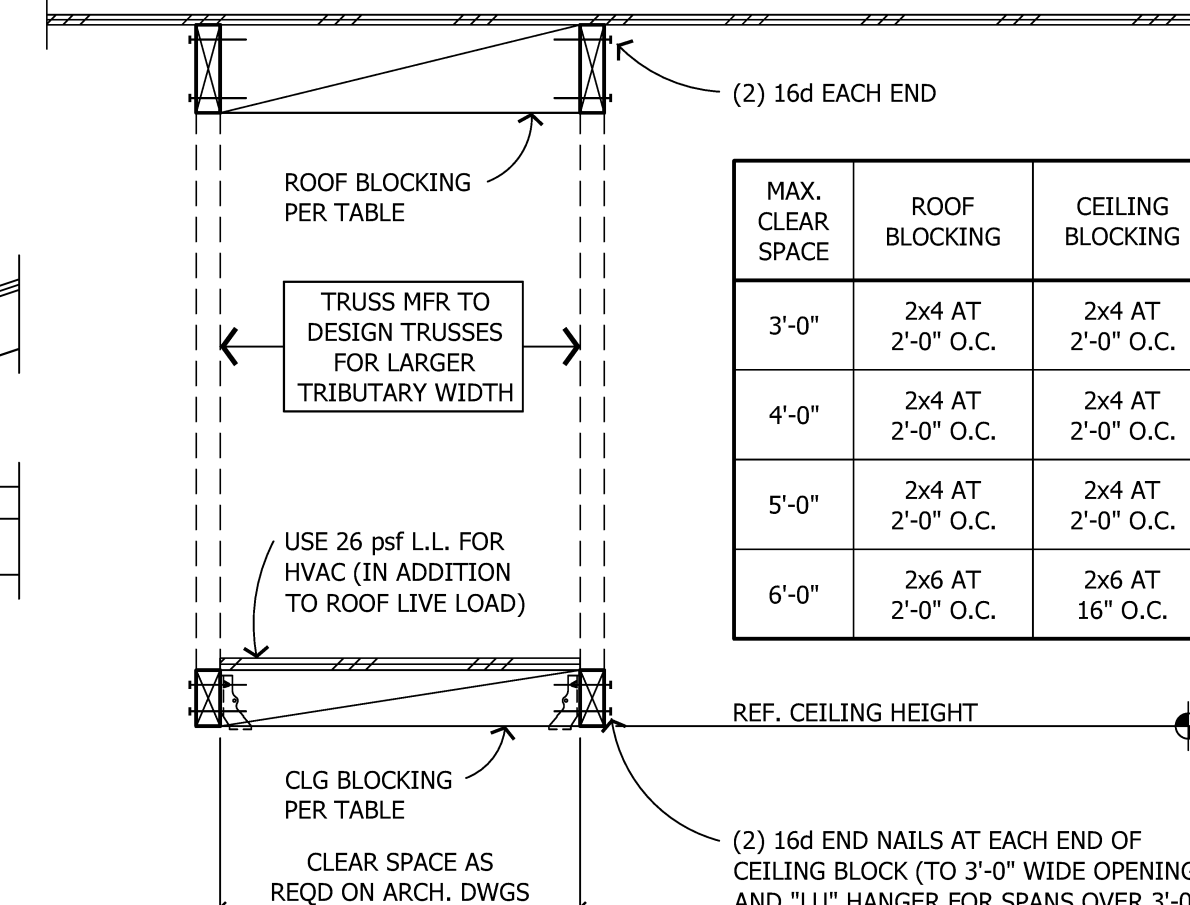


408 FABRICATED TRUSS BLOCKING PANEL ELEVATION ABOVE SHEARWALL

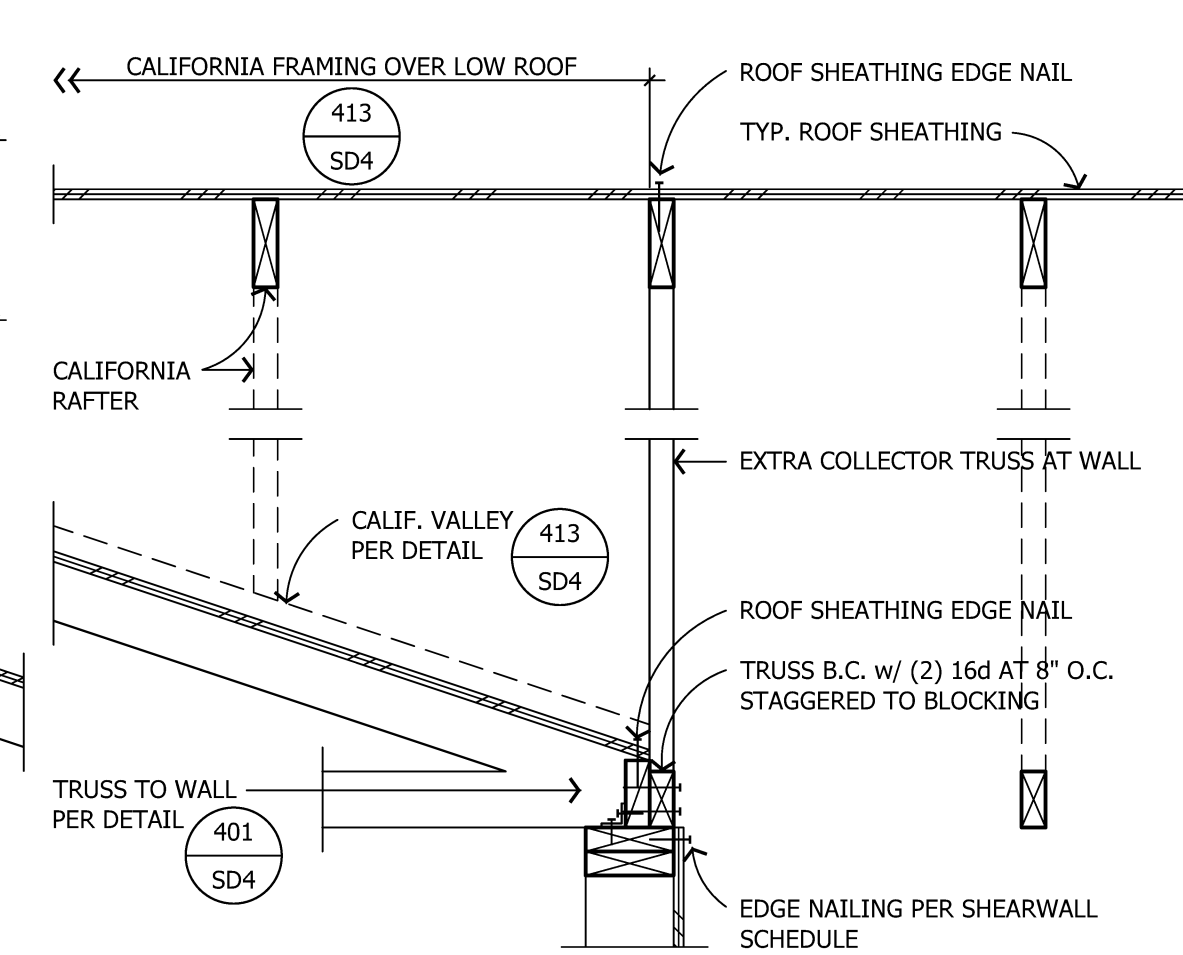


412A OFFSET SHEARWALL

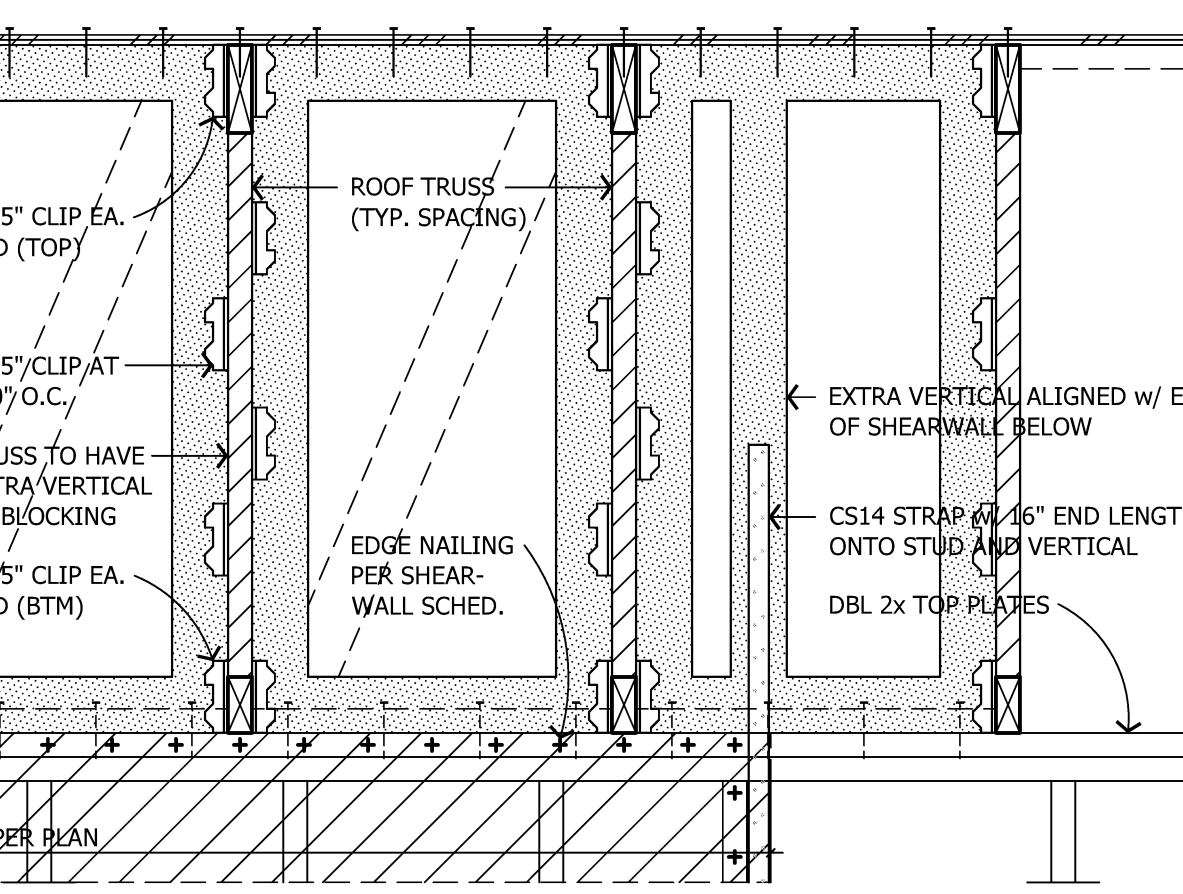
412 PARALLEL TRUSS TO INTERIOR SHEARWALL



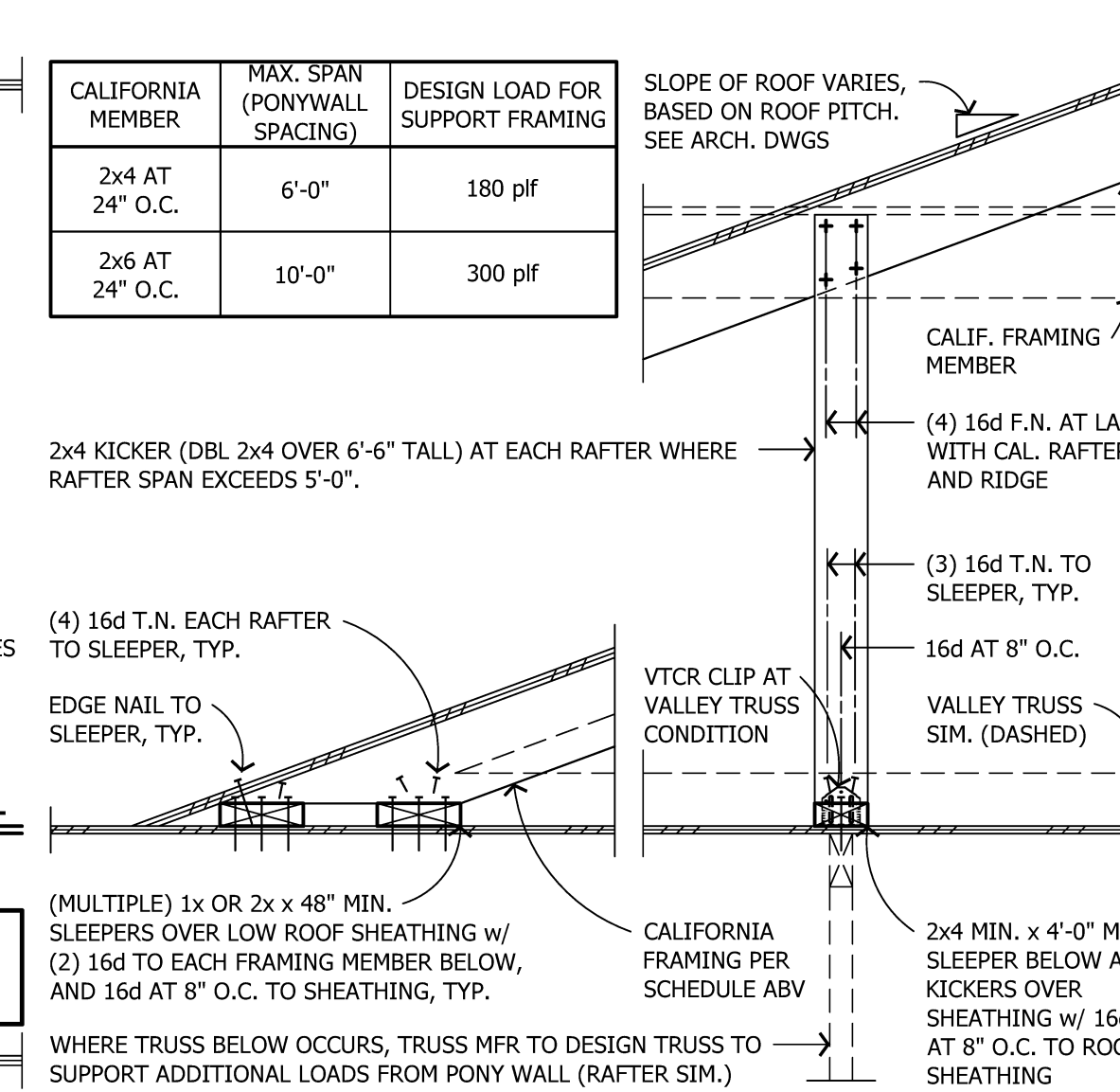
417 ROOF TRUSSES AT HVAC / ATTIC ACCESS



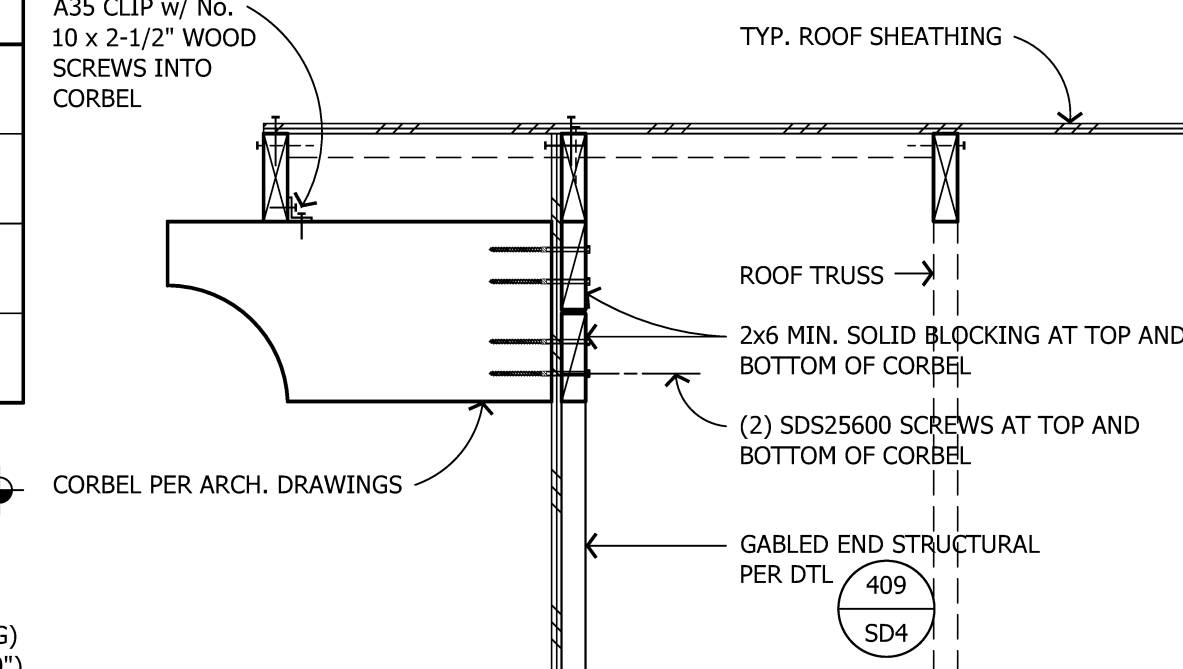
404 TRUSS TO INTERIOR BEARING WALL



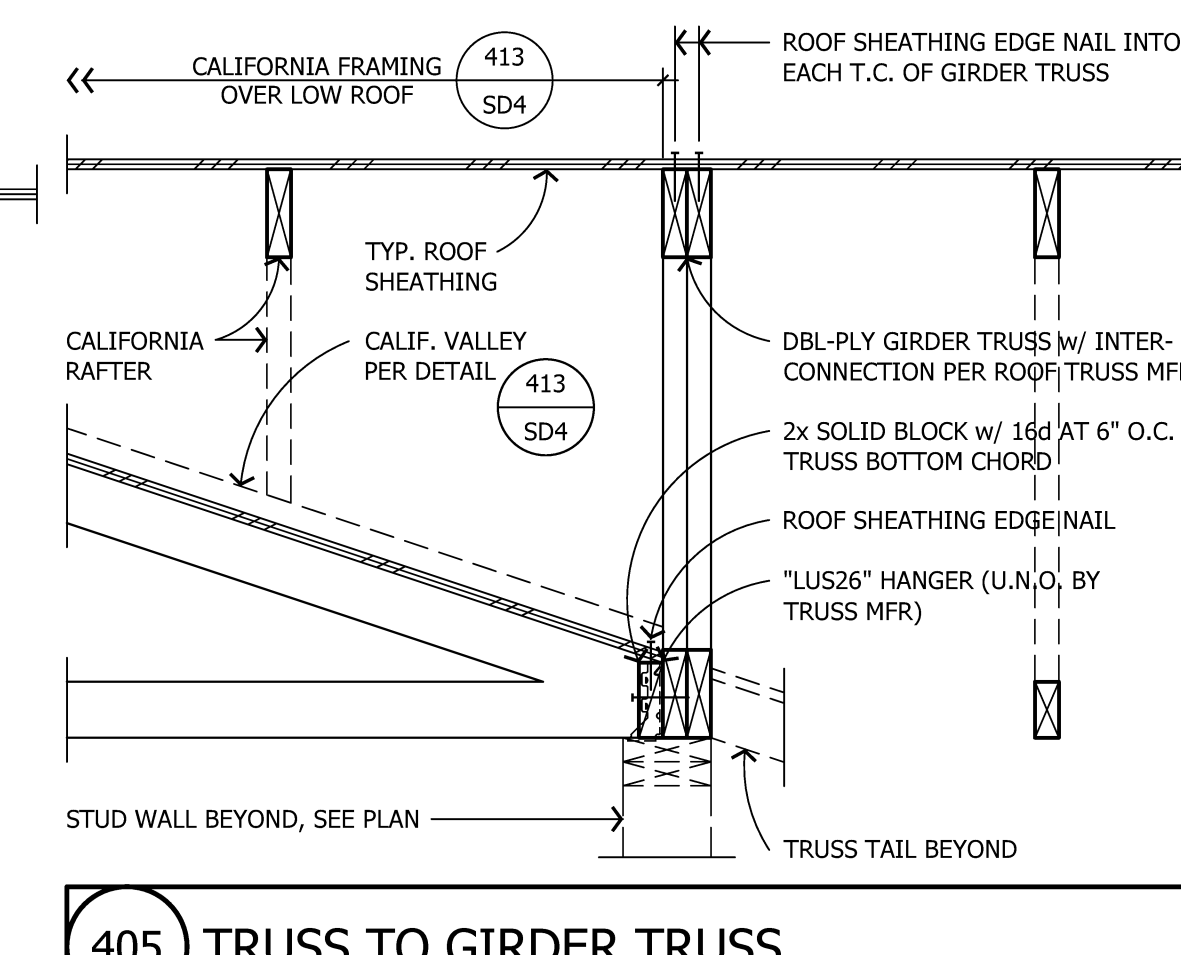
409 GABLED END TRUSS



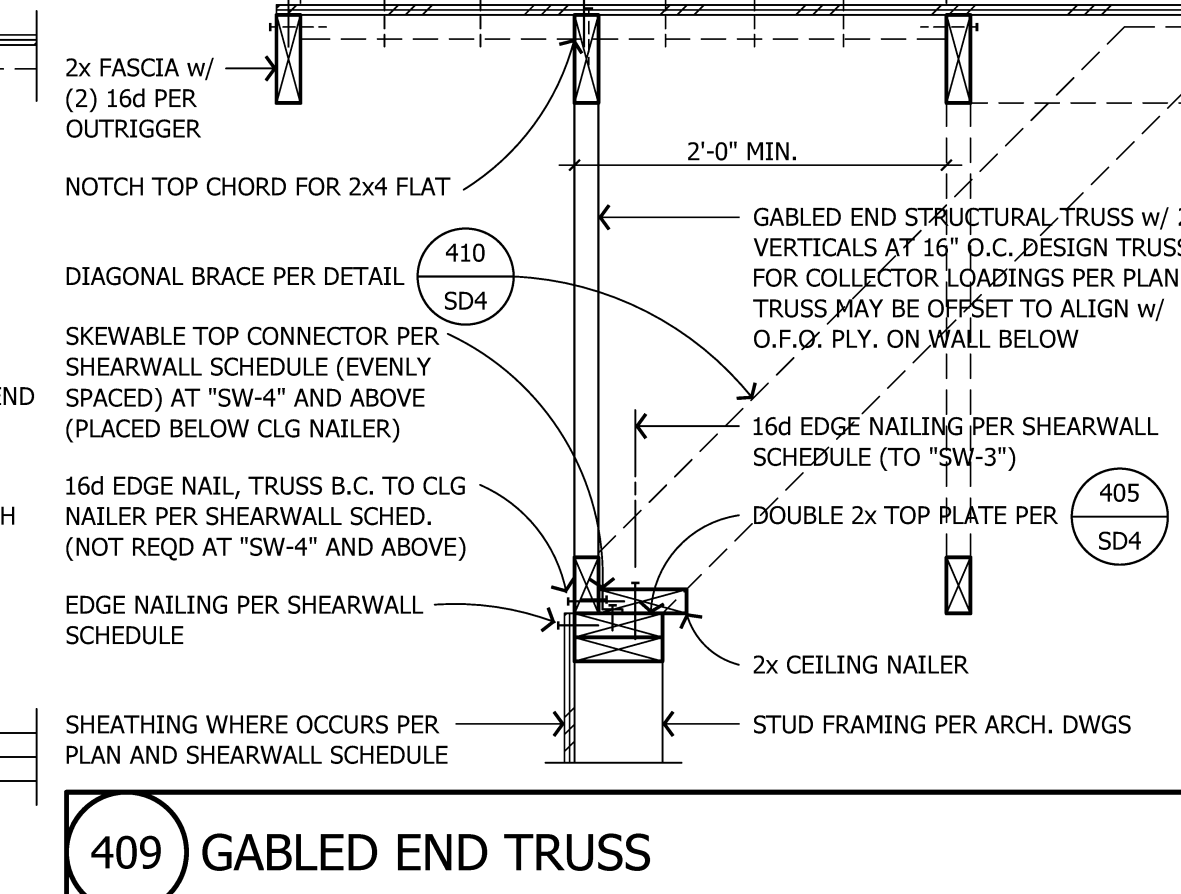
413 CALIFORNIA FRAMING DETAIL



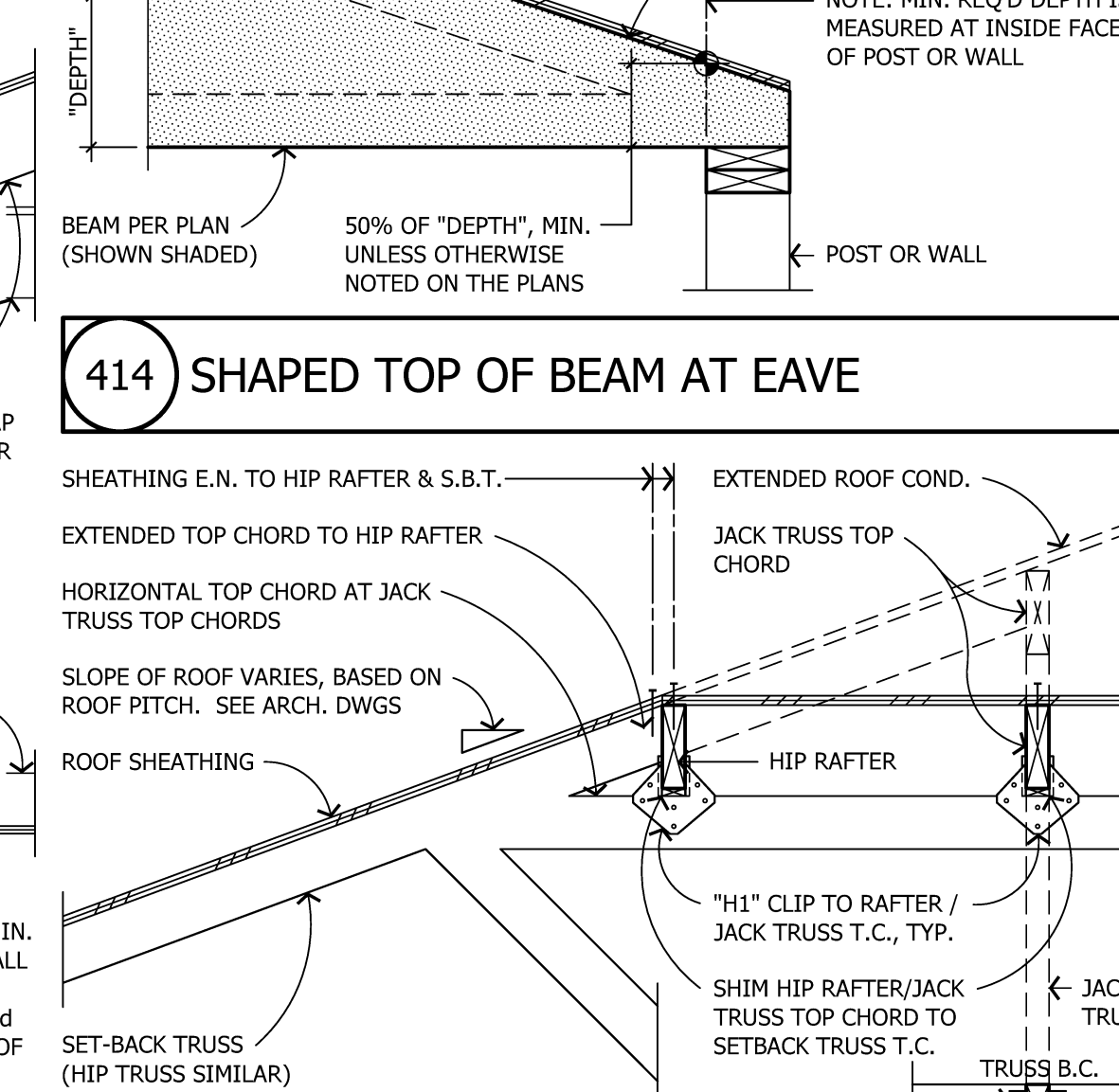
418 CORBEL BEAM AT GABLED END TRUSS



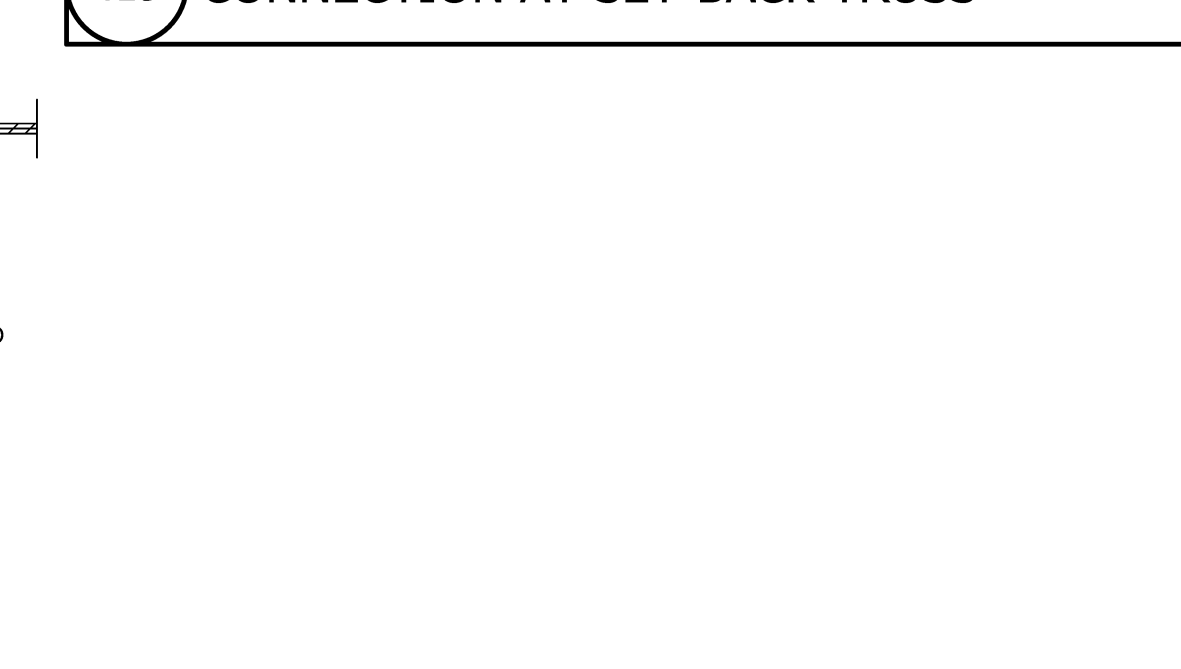
405 TRUSS TO GIRDER TRUSS



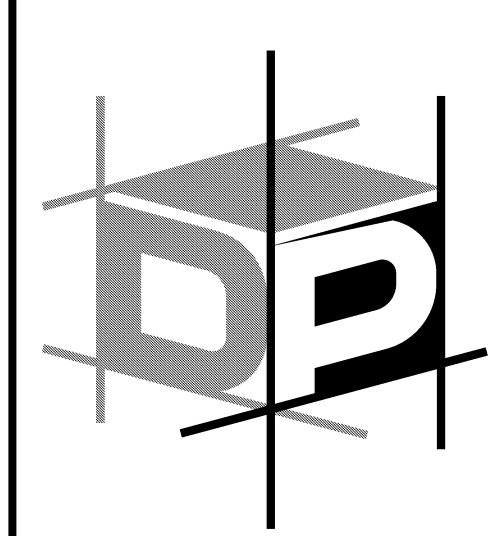
409 GABLED END TRUSS



414 SHAPED TOP OF BEAM AT EAVE



419 CONNECTION AT SET-BACK TRUSS



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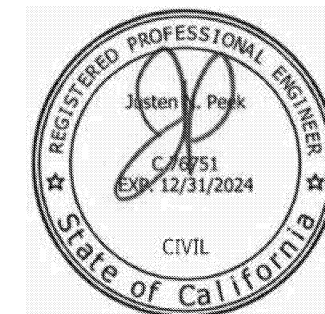
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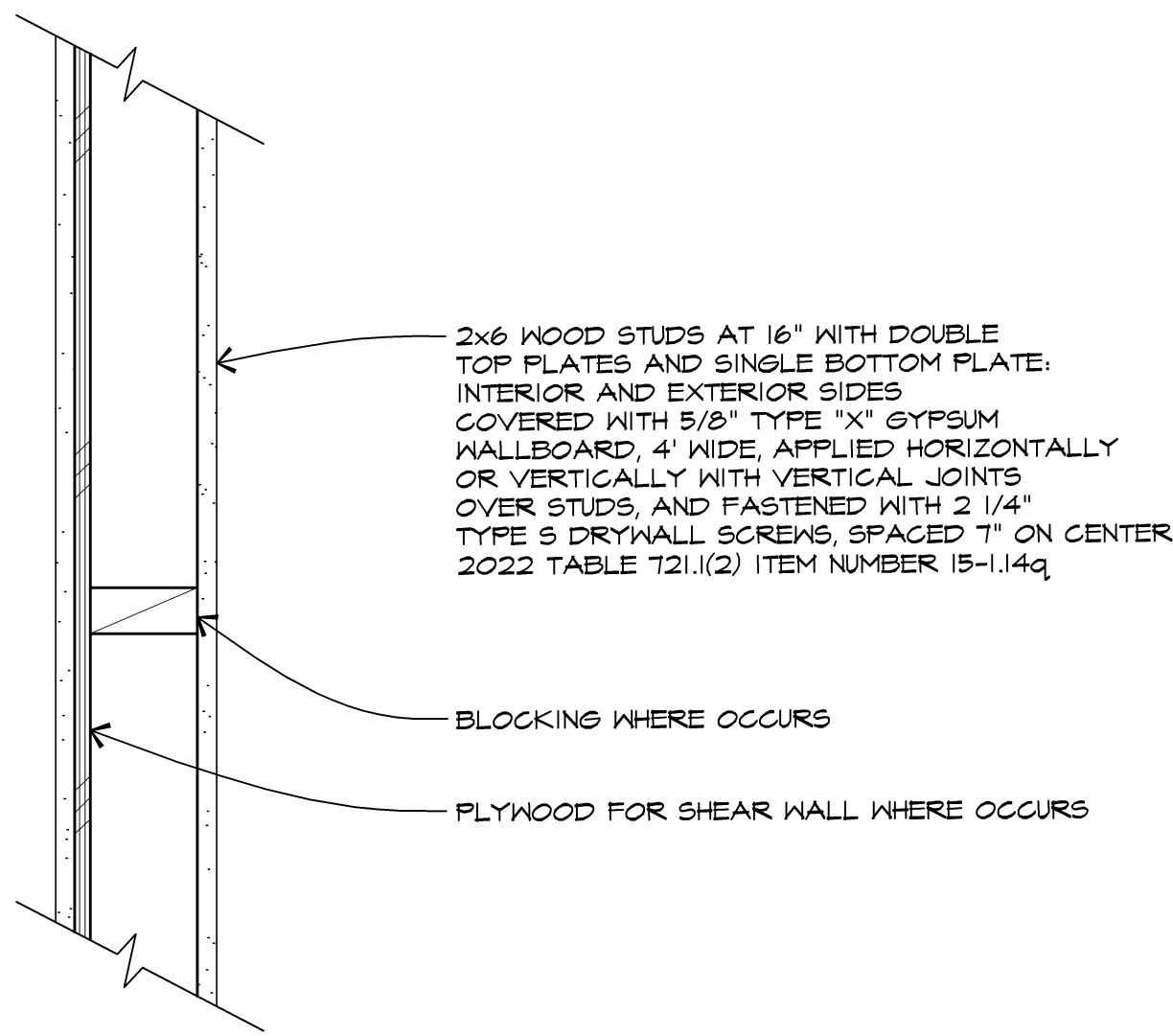
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Sheet Description:
ROOF FRAMING DETAILS

Scale: 1" = 1'-0"
Initial Issue Date: March 24, 2023
Drawn By: E. Bennett
Project Designer: J. Peek
Project Manager: J. Peek Ext. 23
Job No. W021420
Sheet No.

SD4



1
A1
1 HOUR FIRE RATED WALL DETAIL

FLOOR PLAN NOTES:

- 1) ALL NEW EXTERIOR WALLS TO BE 2x6 FRAMED STUD WALLS.
- 2) WINDOWS MARKED W/ (T) SHALL BE TEMPERED SAFETY GLASS.
- 3) ALL WINDOW HEAD HTS. SHALL BE 8'-0" TYP., U.N.O.
- 4) ALL WINDOWS MARKED "EGRESS" SHALL MEET MIN. EMERGENCY EGRESS WINDOW STANDARDS OF MIN. NET CLEAR OPENING OF 5.7 SQ. FT. & MIN. NET CLEAR HEIGHT OF 24" & MIN. NET CLEAR WIDTH OF 34" CONTRACTOR SHALL VERIFY W/ WINDOW MANUFACTURE.
- 5) THERE MUST BE AT LEAST 15" INCHES BETWEEN THE CENTER OF PLUMBING FIXTURES SUCH AS WATER CLOSETS OR SINKS AND THE NEAREST WALL OR OBJECT.
- 6) CONTRACTOR SHALL VERIFY LOCATION & NUMBER OF A/C UNITS W/ OWNER. PROVIDE 4" CONC. SLAB W/ #10 W/M. ALL NECESSARY PLUMBING & ELECTRICAL.
- 7) MINIMUM OF 36-INCH DEEP LANDING IS REQUIRED IN DIRECTION OF TRAVEL. MAXIMUM RISE OF STEP IS 7.75-INCH.
- 8) BATHTUB AND SHOWER FLOORS AND WALLS ABOVE BATHTUBS WITH INSTALLED SHOWER HEADS AND IN SHOWER COMPARTMENTS SHALL BE FINISHED WITH NONABSORBENT SURFACE. SUCH WALL SURFACES SHALL EXTEND TO A HEIGHT OF NOT LESS THAN 6 FEET ABOVE THE FLOOR. CRC R307.2.
- 9) GYPSUM BOARD SHALL NOT BE USED WHERE THERE WILL BE DIRECT EXPOSURE TO WATER, OR IN AREAS SUBJECT TO CONTINUOUS HIGH HUMIDITY. CRC T02.3.7.

*** EGRESS WINDOW DETAIL**

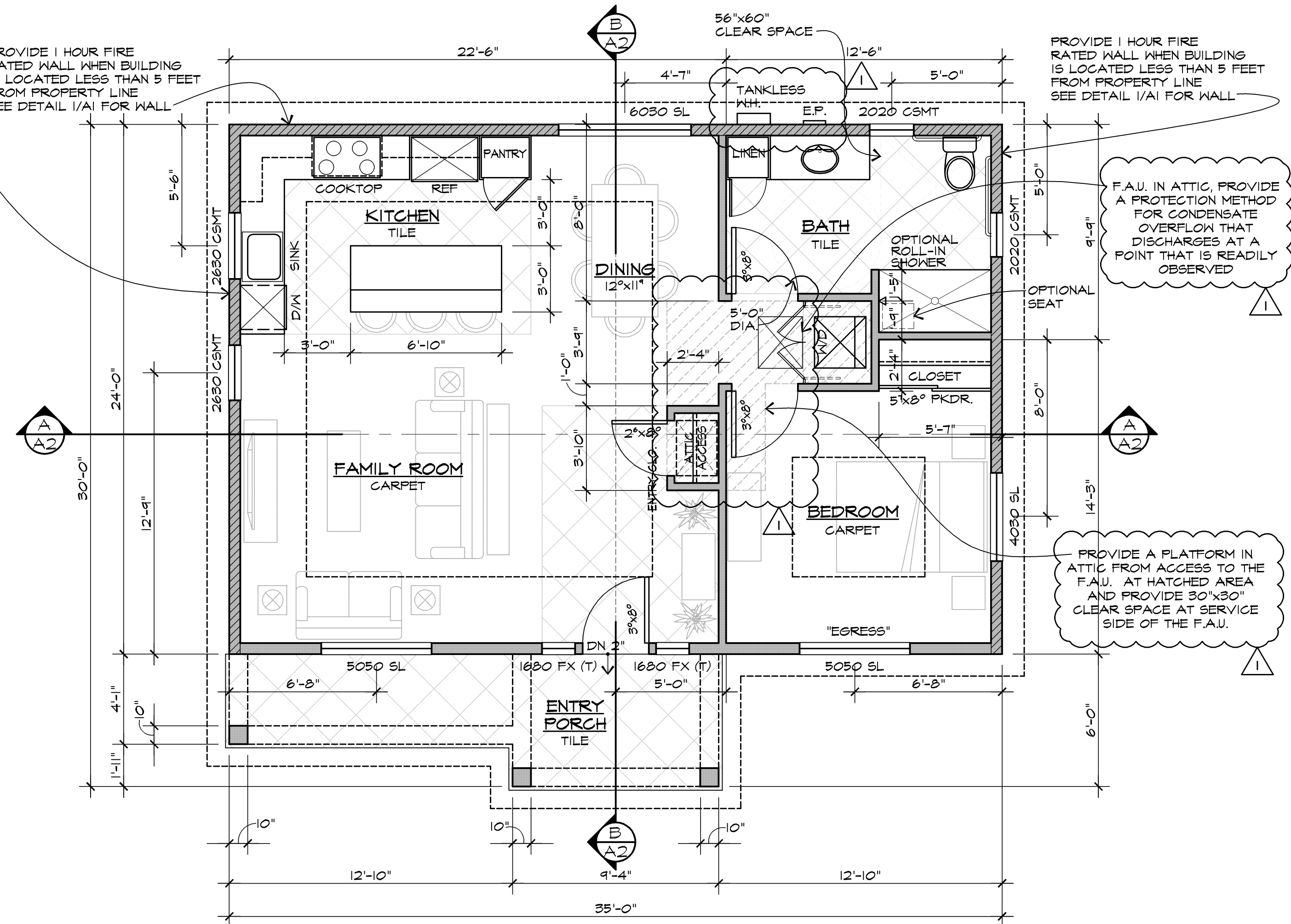
EGRESS WINDOWS SHALL COMPLY W/ ALL 4 CONDITIONS ILLUSTRATED BELOW.

4. MIN. NET CLEAR OPENABLE AREA SHALL BE 5.7 SQ.FT.

42"x20" = 840"/144 = 5.8 SQ.FT.

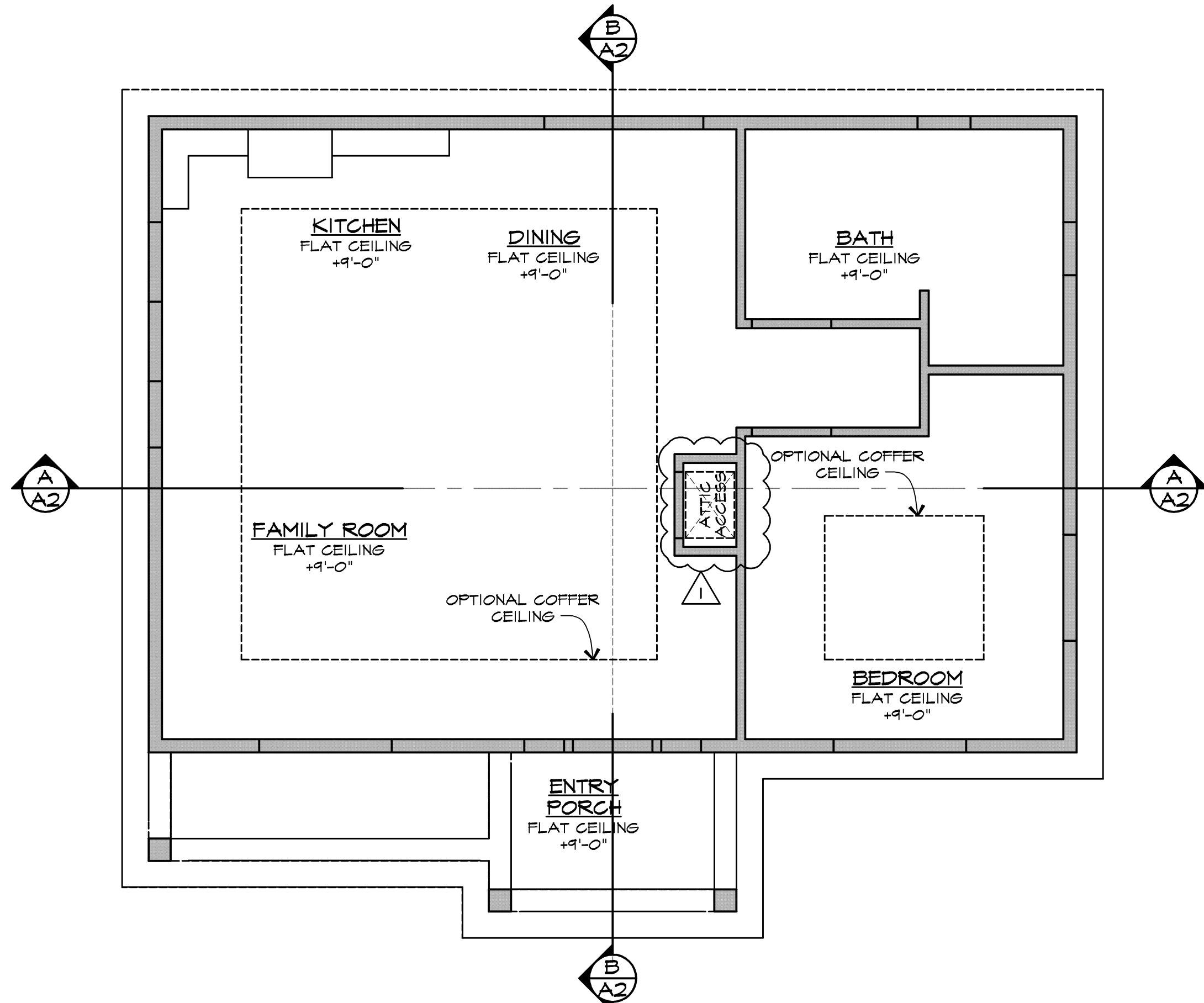
ESCAPE OR RESCUE WINDOWS SHALL HAVE A MINIMUM NET CLEAR OPENABLE AREA OF 5.7 SQ.FT. (0.53 M²). (EXCEPTION: THE MINIMUM NET CLEAR OPENING FOR EMERGENCY ESCAPE AND RESCUE GRADE-FLOOR OPENINGS SHALL BE 5 SQUARE FEET (0.46M²). THE MINIMUM NET CLEAR OPENABLE HEIGHT DIMENSION SHALL BE 24 INCHES (610 MM). THE MINIMUM NET CLEAR OPENABLE WIDTH DIMENSION SHALL BE 20 INCHES (508 MM). THE NET CLEAR OPENING DIMENSIONS SHALL BE THE RESULT OF NORMAL OPERATION OF THE OPENING WHEN WINDOWS ARE PROVIDED AS A MEANS OF ESCAPE OR RESCUE, THEY SHALL HAVE A FINISHED SILL HEIGHT NOT MORE THAN 44 INCHES (1118 MM) ABOVE THE FLOOR. CRC SECTION R310.1, R310.1.1, R310.1.2., R310.1.3. AND R310.1.4.

CONTRACTOR NOTE: CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLIANCE WITH ALL EGRESS REQUIREMENTS.



FLOOR PLAN (MEDITERRANEAN STYLE)

SCALE: 1/4" = 1'-0"



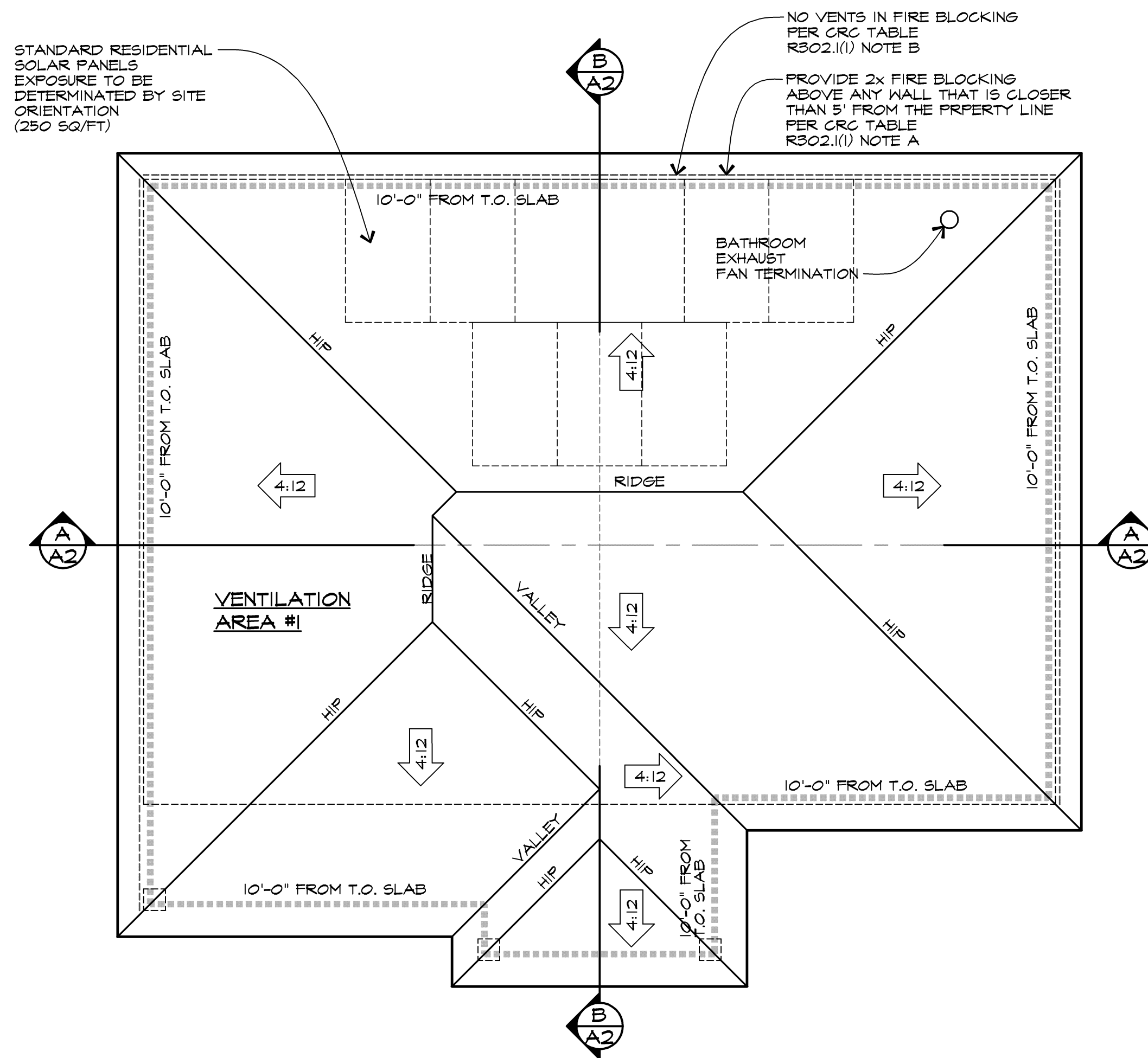
REFLECTED CEILING PLAN

SCALE: 1/4" = 1'-0"

VENTILATION CALCULATIONS

AREA I

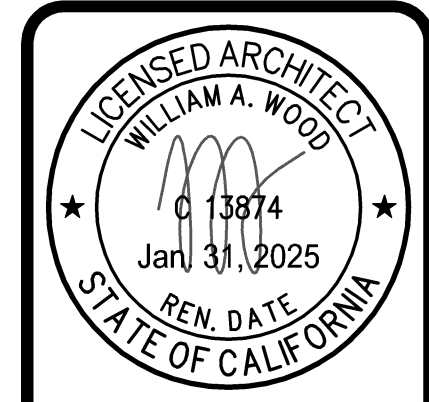
- * ATTIC AREA: 948 SQ. FT.
- * REQUIRED AREA: 455 SQ. IN. (CONVERTED TO INCHES @ 1/300)
- * UPPER AREA: 227.5 SQ. IN. (50% REQUIRED IN UPPER 1/3)
- * PROVIDE 227.5 SQ. IN. IN UPPER 1/3 OFF ROOF WITH OHAGIN'S "FLAT" VENT (98.75 SQ. IN. FREE AREA) * 2 VENTS REQUIRED
- * LOWER AREA: 227.5 SQ. IN. (50% REQUIRED IN LOWER 1/3)
- * TO BE PROVIDED BY 32.5 VENTED FRIEZE BLOCKS MIN. (7 SQ. IN. FREE VENTING PER BLOCK VIA 3-2" DIA. HOLES)



ROOF PLAN

SCALE: 1/4" = 1'-0"

REVISIONS	DATE
PLAN CHECK COMMENTS	4-25-23



DANVILLE ACCESSORY
DWELLING UNITS - ONE BEDROOM

DANVILLE CA 94526

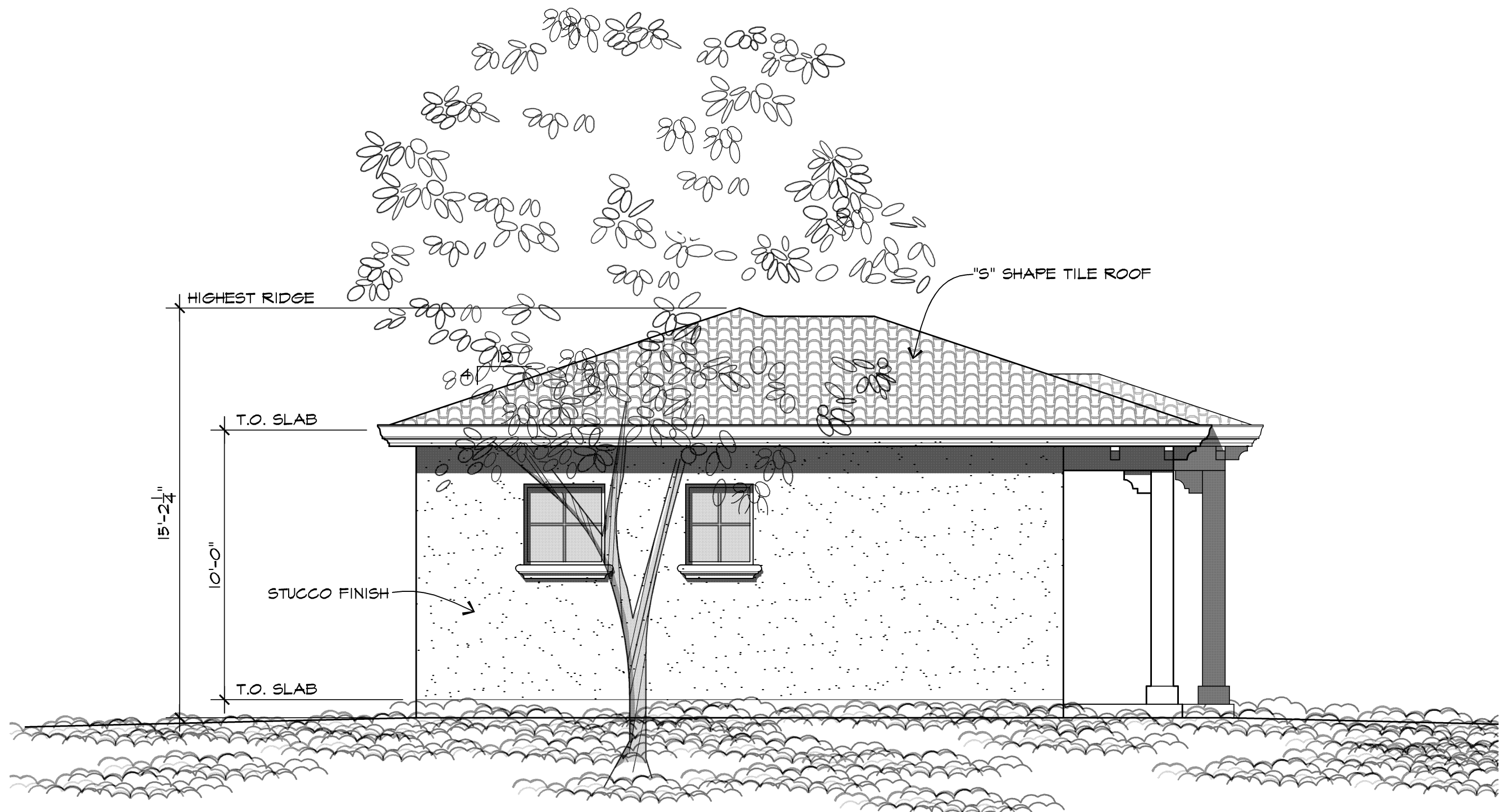
WILLIAM WOOD
ARCHITECTS

301 HARTZ AVENUE, SUITE 203
DANVILLE, CALIFORNIA 94526
(925) 820-8233

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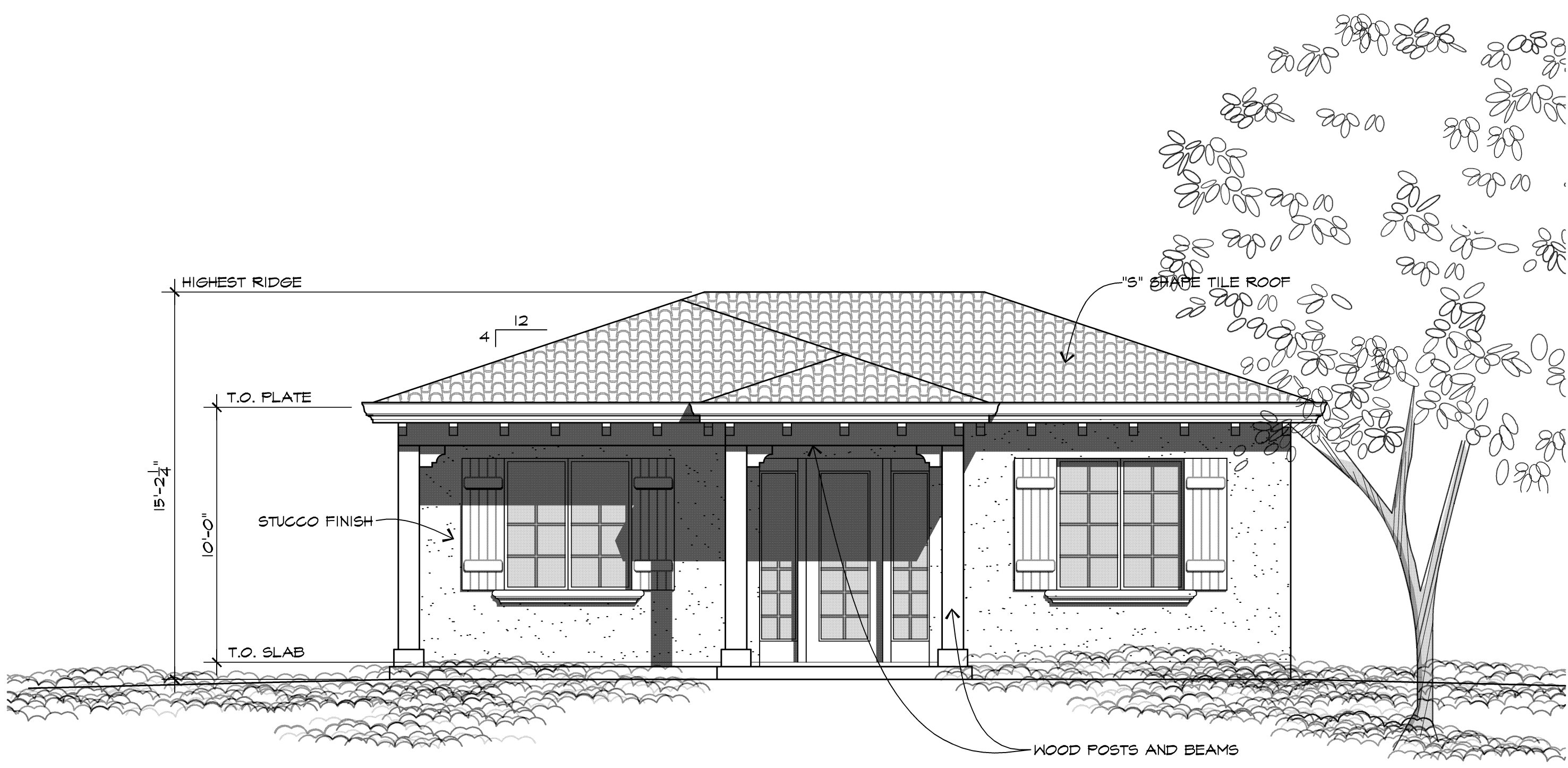
DRAWN JJ
CHECKED WW
DATE 03-30-23
SCALE AS SHOWN
JOB NO. 20.921R
SHEET A2
OF SHEETS

Apr 26, 2025 - 2:21pm A2 One Bedroom Mediterranean Style.dwg



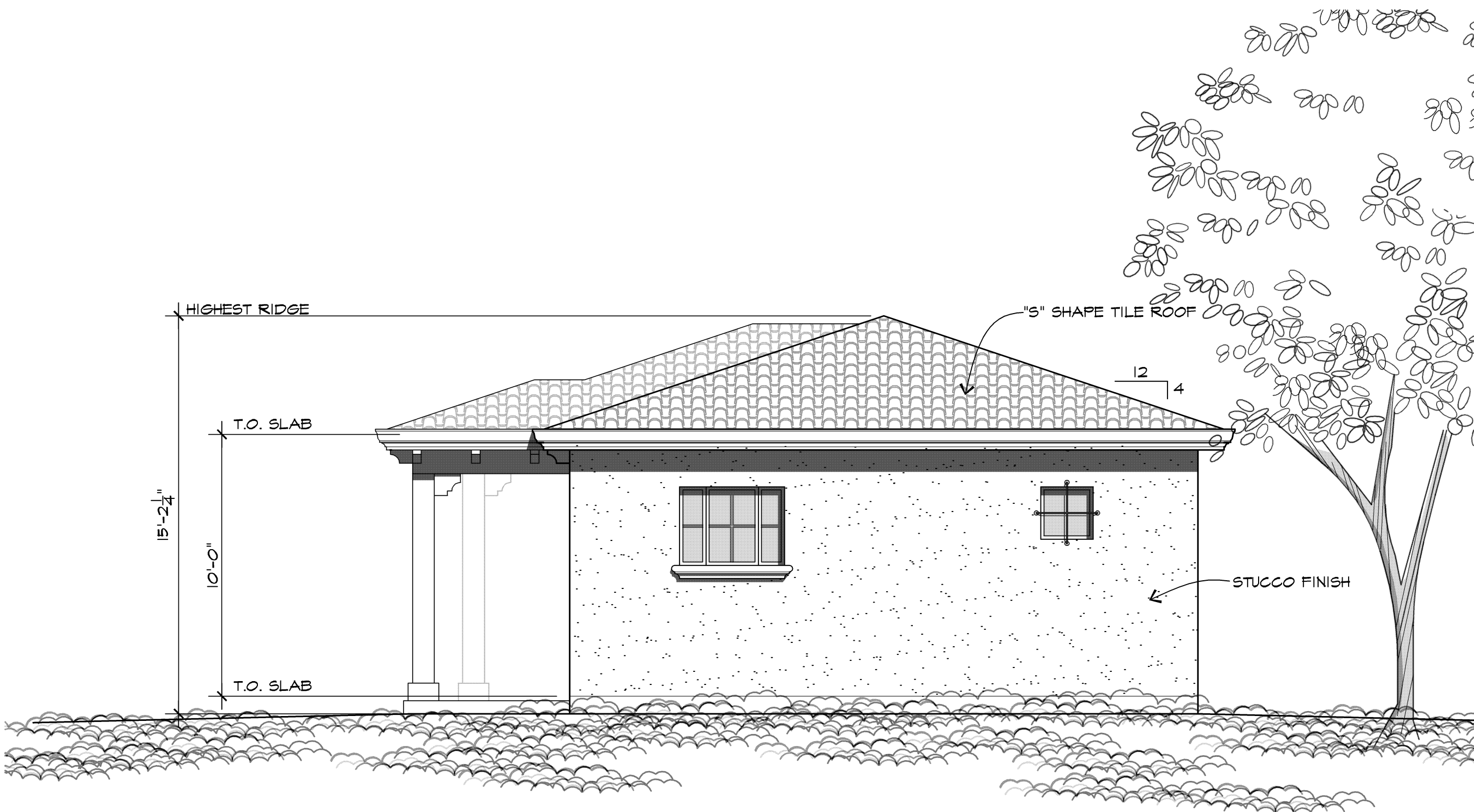
LEFT SIDE ELEVATION - (MEDITERRANEAN STYLE)

SCALE: 1/4" = 1'-0"



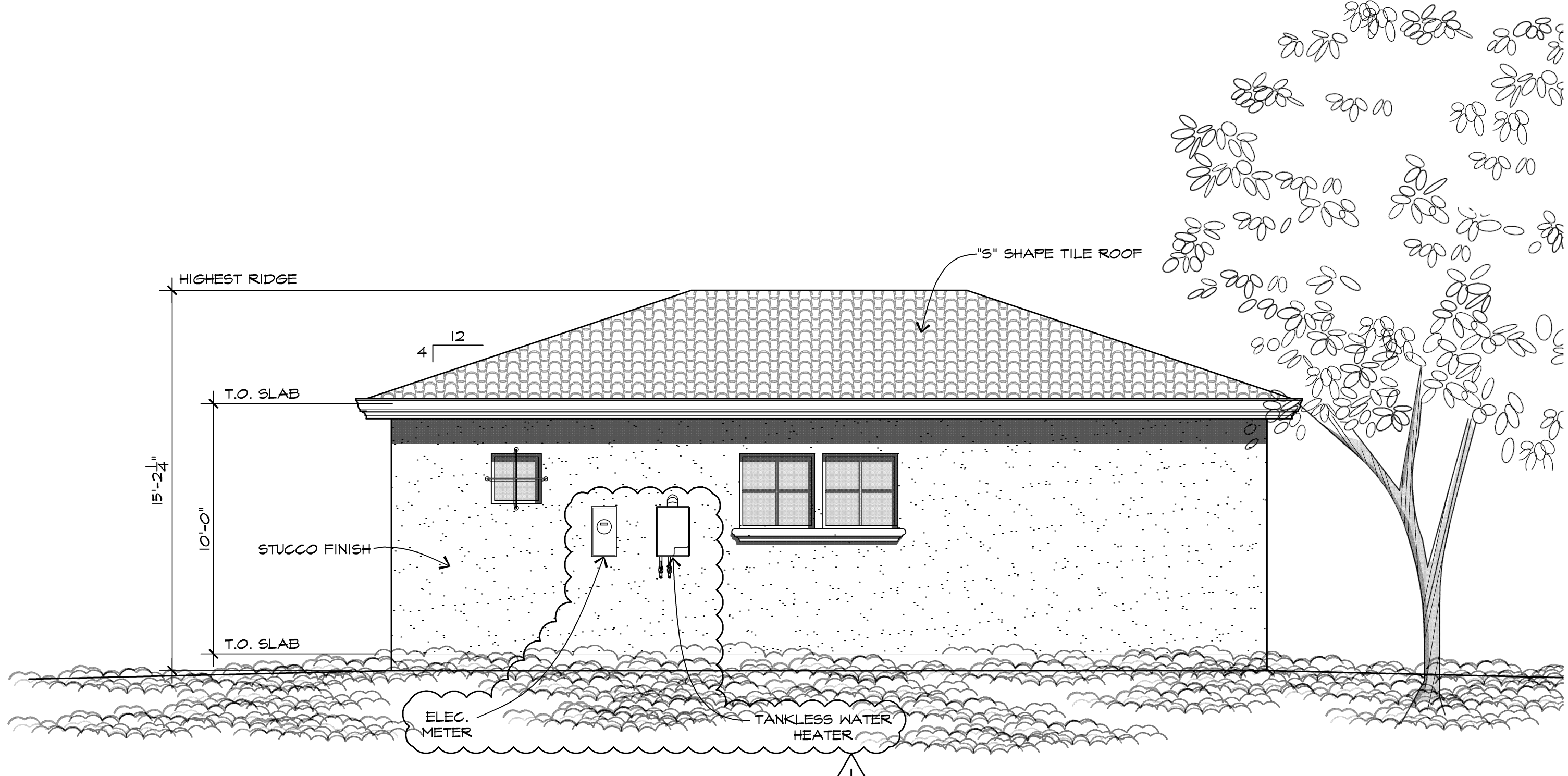
FRONT ELEVATION - (MEDITERRANEAN STYLE)

SCALE: 1/4" = 1'-0"



RIGHT SIDE ELEVATION - (MEDITERRANEAN STYLE)

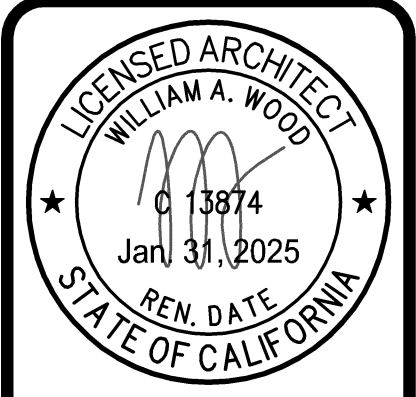
SCALE: 1/4" = 1'-0"



REAR ELEVATION - (MEDITERRANEAN STYLE)

SCALE: 1/4" = 1'-0"

REVISIONS		DATE
△	PLAN CHECK COMMENTS	4-25-23
△		
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△		



DANVILLE ACCESSORY
DWELLING UNITS - ONE BEDROOM

DANVILLE CA 94526

**WILLIAM WOOD
ARCHITECTS**
301 HARTZ AVENUE, SUITE 203
DANVILLE, CALIFORNIA 94526
(925) 820-8233

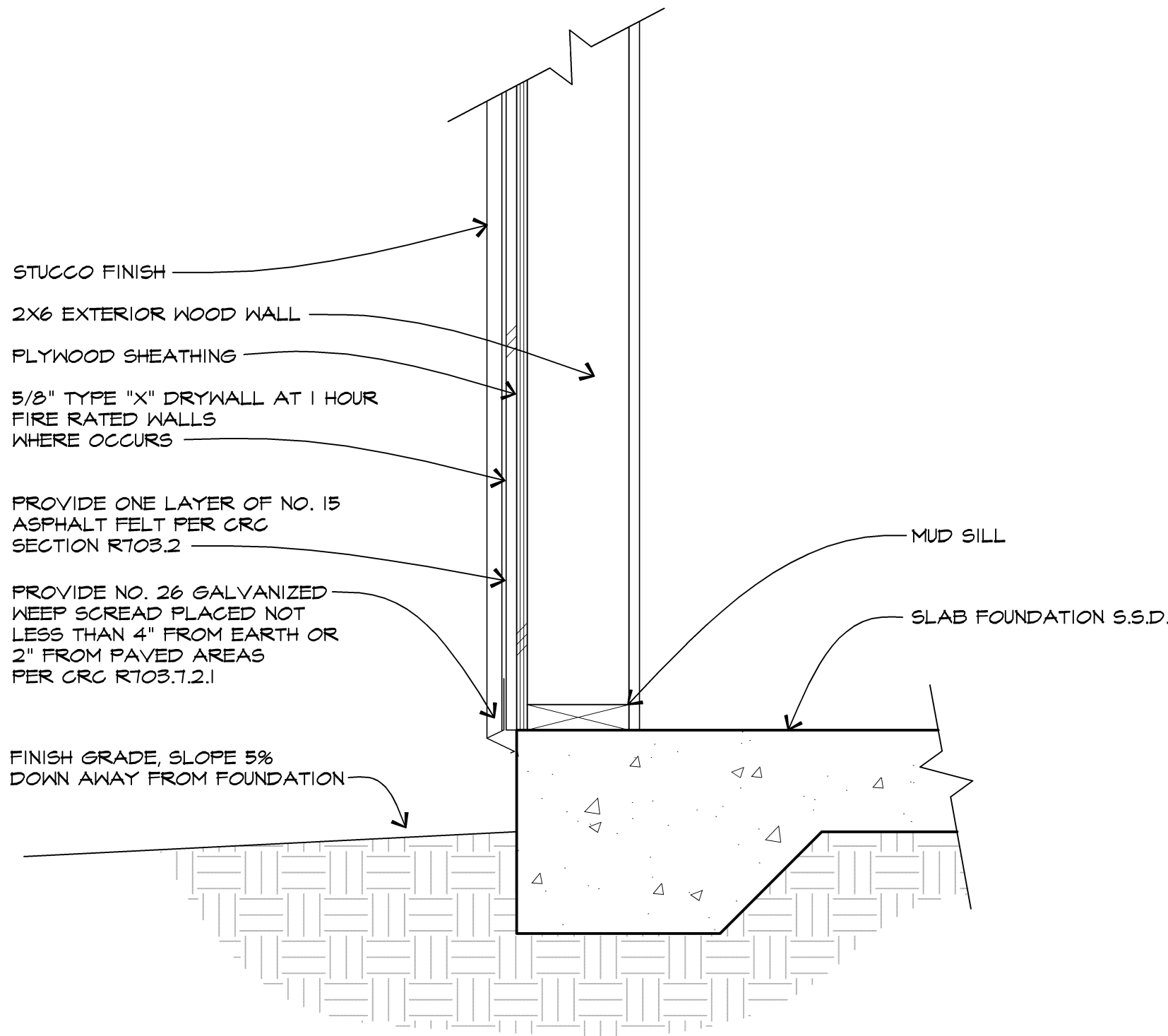
THIS IS AN ORIGINAL, UNPUBLISHED
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DRAWN JJ
CHECKED WW
DATE 03-30-23
SCALE AS SHOWN
JOB NO. 20.921R
SHEET

A3

OF SHEETS

Apr 26, 2025 - 5:25pm A3 One Bedroom Mediterranean Style.dwg

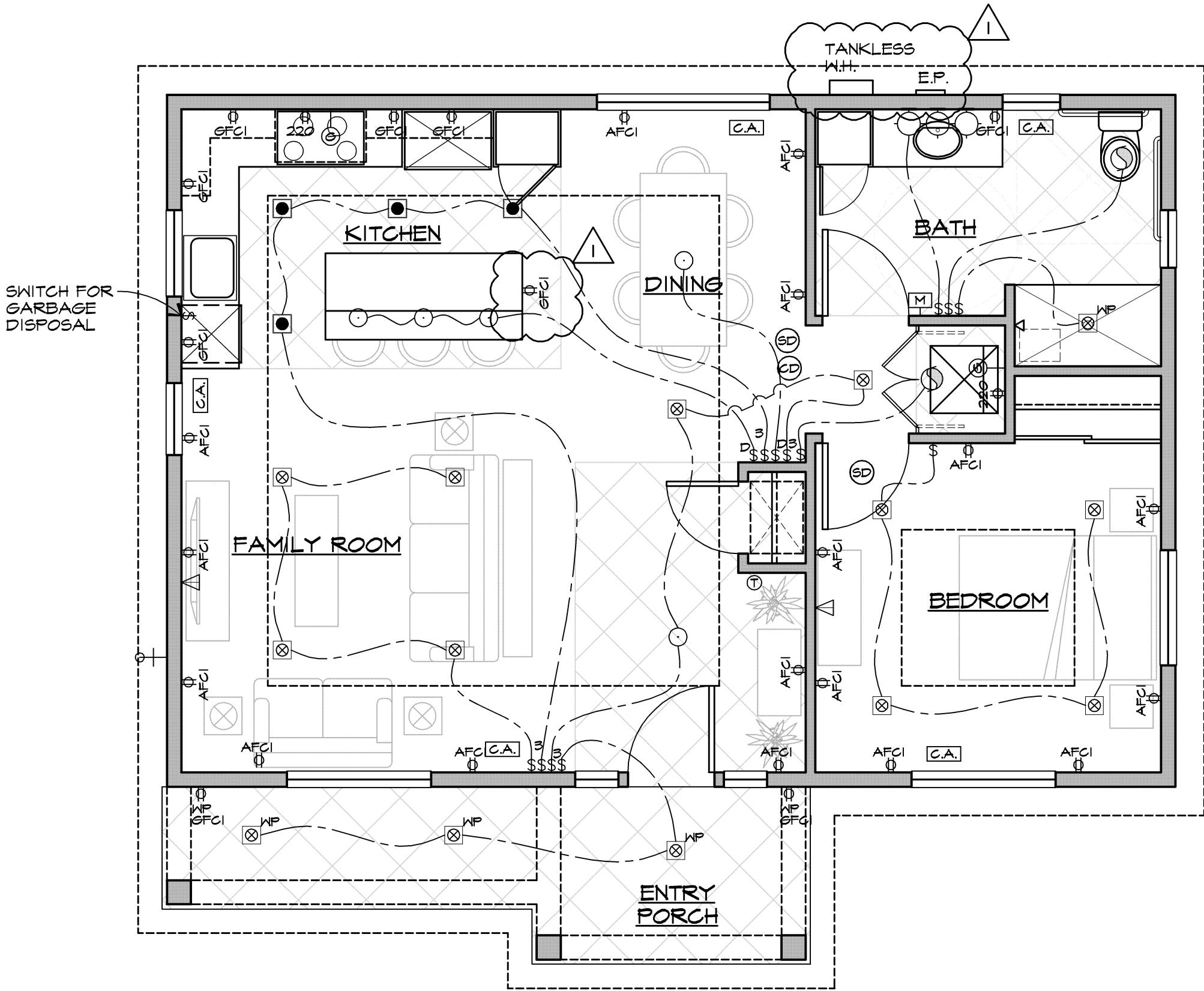


TYPICAL WALL DETAIL

SCALE: 1 1/2" = 1'-0"

- ELECTRICAL/MECHANICAL KEY**
- SWITCHED RECEPTACLE OUTLET @ 12" U.N.O.
 - DUPLEX RECEPTACLE OUTLET @ 12" U.N.O. W/ ARC-FAULT CIRCUIT INTERRUPTER
 - DUPLEX RECEPTACLE OUTLET W/ GROUND FAULT CIRCUIT INTERRUPTER
 - WEATHER PROOF DUPLEX RECEPTACLE OUTLET (GFCI PROTECTED PER NEC210-8(g)(5))
 - DUPLEX RECEPTACLE OUTLET - 220 VOLT
 - SINGLE POLE SWITCH @ 48" U.N.O.
 - THREE-WAY SWITCH
 - SWITCH W/ DIMMER
 - SWITCH W/ MOTION LIGHT
 - MOTION SENSOR
 - CEILING MOUNTED LIGHT FIXTURE
 - WALL MOUNTED LIGHT FIXTURE
 - WALL MOUNTED LIGHT FIXTURE - SHALL BE SUITABLE FOR DAMP LOCATIONS
 - HANGING LIGHT
 - RECESSED LED CEILING LIGHT FIXTURE
 - RECESSED LED LOW VOLTAGE CEILING LIGHT
 - RECESSED LIGHT FIXTURE - SHALL BE LABELED SUITABLE FOR DAMP LOCATIONS
 - EXHAUST FAN
 - SMOKE DETECTOR - (110V W/ BATTERY BACK-UP) HARD WIRED & INTERCONNECTED
 - CARBON MONOXIDE DETECTOR - (110V W/ BATTERY BACK-UP) HARD WIRED & INTERCONNECTED
 - TELEVISION OUTLET
 - HOSE BIBB (1/2" CW) W/ NON-REMOVABLE ANTI-SIPHON DEVICE (FOR OUTSIDE BIBB)
 - GAS OUTLET
 - THERMOSTAT
 - CEILING SUPPLY AIR REGISTER
 - FLOOR AIR SUPPLY REGISTER
 - COLD AIR RETURN @ CEILING

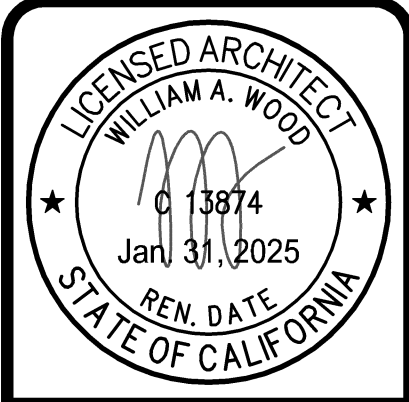
- ELECTRICAL NOTES**
- THERMOSTAT LOCATION TO BE DETERMINED BY HVAC DESIGNER.
 - CEILING AIR REGISTERS TO BE DETERMINED BY HVAC DESIGNER.
 - KITCHEN & BATHROOM LIGHTING SHALL MEET EFFICIENCY REQUIREMENT OF 2022 TITLE 24.
 - CONTRACTOR TO PROVIDE A COLD WATER CONNECTION @ THE REFRIGERATOR.
 - CONTRACTOR TO PROVIDE THE OPTION OF GAS OR 220V @ THE COOK TOP (VERIFY W/ OWNER).
 - WATER CLOSETS TO BE MAX 1.28 GALLONS PER FLUSH PER CAL GREEN SECTION 4.303.1.1.
 - INSTALLATION INSTRUCTIONS FOR ALL EQUIP. SHALL BE PROVIDED TO THE FIELD INSPECTOR @ THE TIME OF INSPECTION.
 - PROVIDE TWO SMALL APPLIANCE BRANCH CIRCUITS FOR THE KITCHEN AND ARE LIMITED TO SUPPLYING WALL AND COUNTER SPACE OUTLETS FOR THE KITCHEN, PANTRY, BREAKFAST ROOM, DINING ROOM, OR SIMILAR AREAS. NOTE: THESE CIRCUITS CANNOT SERVE OUTSIDE PLUGS, RANGE HOOD, DISPOSALS, DISHWASHERS OR MICROWAVES-ONLY THE REQUIRED COUNTERTOP/WALL OUTLETS INCLUDING THE REFRIGERATOR.
 - PROVIDE A DEDICATED 20-AMP BRANCH CIRCUIT TO SUPPLY THE LAUNDRY RECEPTACLE OUTLET.
 - PROVIDE AN ARC-FAULT CIRCUIT INTERRUPTER TO PROTECT ALL RECEPTACLES IN THE BEDROOMS.
 - PROVIDE RECEPTACLE OUTLETS IN FOLLOWING LOCATIONS:
 - A. AT EACH KITCHEN AND DINING AREA COUNTER SPACE WIDER THAN 12" LOCATED SO THAT NO POINT ALONG THE COUNTER WALL IS OVER 24" FROM A RECEPTACLE.
 - B. AN EXTERIOR RECEPTACLES AT THE FRONT AND REAR OF THE HOME. THEY MUST BE WITHIN 6 FEET, 6 INCHES OF GRADE AND WATERPROOF.
 - C. ONE RECEPTACLE FOR THE LAUNDRY, (I.E. WASHER).
 - D. IN ANY HALLWAY 10 FT OR MORE IN LENGTH.
 - KITCHEN COUNTERTOP RECEPTACLES SHALL BE LOCATED ABOVE THE COUNTERTOP NOT MORE THAN 18", AND MAY NOT BE INSTALLED FACE-UP IN THE COUNTER.
 - ALL LIGHT FIXTURE IN TUB OR SHOWER ENCLOSURES SHALL BE LABEL "SUITABLE FOR DAMP LOCATIONS" AND IF CEILING HEIGHT IS > 8 FT. BE PROTECTED BY GFCI.
 - AN APPROVED CARBON MONOXIDE ALARM SHALL BE INSTALLED IN DWELLING UNITS AND IN SLEEPING UNITS WITHIN WHICH FUEL-BURNING APPLIANCES ARE INSTALLED. CARBON MONOXIDE ALARMS REQUIRED BY SECTIONS R315.1 AND R315.2 SHALL BE INSTALLED IN THE FOLLOWING LOCATIONS: OUTSIDE OF EACH SEPARATE DWELLING UNIT SLEEPING ARE IN THE IMMEDIATE VICINITY OF THE BEDROOM(S), ON EVERY LEVEL OF A DWELLING UNIT INCLUDING BASEMENTS. CARBON MONOXIDE ALARMS COMBINED WITH SMOKE ALARMS SHALL COMPLY WITH SECTION R315, ALL APPLICABLE STANDARDS, AND REQUIREMENTS FOR LISTING AND APPROVAL BY THE OFFICE OF THE STATE FIRE MARSHALL, FOR SMOKE ALARMS.
 - EXHAUST FANS TO BE 50 CFM INTERMITTENT OR 20 CFM CONTINUOUS IN WATER CLOSETS, 50 CFM CONTINUOUS.
 - ALL RECEPTACLES IN THE BATHROOMS, UTILITY ROOMS, KITCHEN COUNTERS, KITCHEN ISLAND, GARAGE (INCLUDING CEILING MOUNTED) AND ALL EXTERIOR LOCATIONS ARE TO BE GFCI PROTECTED CEC 210.8(g).
 - CONDUIT IN CONCRETE SLAB IS TO BE RATED FOR WET LOCATIONS. CEC 430.
 - PROVIDE DEDICATED CIRCUITS FOR FOR BATHROOM CIRCUITS SERVING ONLY THE RECEPTACLES PER CEC201.11(g)(5).
 - PROVIDE TAMPER RESISTANT RECEPTACLES IN ALL NEW CIRCUITS PER CEC SECTION 406.12, & 406.4(D)(5).
 - OUTDOOR LIGHTS TO BE "HIGH EFFICACY" OR "LOW EFFICACY" AND ON PHOTO/MOTION SENSOR PER CA ENERGY SECTION 150.0(K)(4).
 - HOSE BIBS SHALL HAVE NON-REMOVABLE BACKFLOW PREVENTER OR BIB-TYPE VACUUM BREAKER.
 - WATER CLOSETS TO BE 1.28 GALLONS PER FLUSH MAXIMUM OR DUAL FLUSH PER CPC 411.2.
 - KITCHEN FAUCET TO BE 1.8 GALLONS PER MINUTE, MAXIMUM PER CPC 4.303.1.4.4.
 - LAVATORY FAUCET TO BE 1.2 GALLONS PER MINUTE, MAXIMUM PER CPC 467.2.2.
 - SHOWER HEADS TO BE 1.8 GALLONS PER MINUTE AT 80 PSI, MAXIMUM, PER CPC 403.3.



ELECTRICAL/ MECHANICAL FLOOR PLAN

SCALE: 1/4" = 1'-0"

REVISIONS	DATE
PLAN CHECK COMMENTS	4-25-23



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DWELLING UNITS - ONE BEDROOM

DANVILLE CA 94526

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Apr 26, 2025 - 2:22pm A4 One Bedroom Mediterranean Style.dwg

DRAWN	JJ
CHECKED	WW
DATE	03-30-23
SCALE	AS SHOWN
JOB NO.	20.921R
SHEET	
A4	
OF	SHEETS

BUILDING ENERGY ANALYSIS REPORT

PROJECT:

Town of Danville ADU
Danville, CA 94526

Project Designer:

William Wood Architects
301 Hartz Ave #203
Danville, CA 94526
(925) 820-8233

Report Prepared by:

Jason Bennett
DP Advanced Engineering Inc.
3381 Walnut Blvd. Suite 220
Brentwood, CA 94513
(925) 516-3502

Job Number:

W021420

Date:

March 31, 2023

CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD
CF1R-PRF-01E
Project Name: Danville ADU Mediterranean 1Bedroom

Calculation Date/Time: 2023-03-30T18:16:58-07:00

(Page 1 of 14)
Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Mediterranean_1Bedroom.ribd22

GENERAL INFORMATION					
01	Project Name	Danville ADU Mediterranean 1Bedroom			
02	Run Title	Title 24 Analysis			
03	Project Location	Varies			
04	City	Danville, CA	05	Standards Version	2022
06	Zip code	94526	07	Software Version	CBECC-Res 2022.2.1
08	Climate Zone	12	09	Front Orientation (deg/ Cardinal)	All orientations
10	Building Type	Single family	11	Number of Dwelling Units	1
12	Project Scope	Newly Constructed	13	Number of Bedrooms	1
14	Addition Cond. Floor Area (ft ²)	0	15	Number of Stories	1
16	Existing Cond. Floor Area (ft ²)	n/a	17	Fenestration Average U-factor	0.32
18	Total Cond. Floor Area (ft ²)	840	19	Glazing Percentage (%)	18.00%
20	ADU Bedroom Count	n/a			

COMPLIANCE RESULTS	
01	Building Complies with Computer Performance
02	This building incorporates features that require field testing and/or verification by a certified HERS rater under the supervision of a CEC-approved HERS provider.
03	This building incorporates one or more Special Features shown below

Registration Number:

223-P010038749A-000-000-0000000-0000

Registration Date/Time:

2023-03-31 08:34:41

HERS Provider:

CalCERTS inc.

CA Building Energy Efficiency Standards - 2022 Residential Compliance

Report Version: 2022.0.000

Schema Version: rev 20220901

Report Generated: 2023-03-30 18:17:46

CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD
CF1R-PRF-01E
Project Name: Danville ADU Mediterranean 1Bedroom

Calculation Date/Time: 2023-03-30T18:16:58-07:00

(Page 2 of 14)
Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Mediterranean_1Bedroom.ribd22

ENERGY DESIGN RATINGS						
	Energy Design Ratings			Compliance Margins		
	Source Energy (EDR1)	Efficiency ¹ EDR (EDR2efficiency)	Total ² EDR (EDR2total)	Source Energy (EDR1)	Efficiency ¹ EDR (EDR2efficiency)	Total ² EDR (EDR2total)
Standard Design	38.1	44.8	38.9			
Proposed Design						
North Facing	28.5	39.5	35.5	9.6	5.3	3.4
East Facing	28.6	40	35.7	9.5	4.8	3.2
South Facing	28.1	38.4	34.8	10	6.4	4.1
West Facing	28.7	41.9	36.9	9.4	2.9	2
RESULT ³ : PASS						
¹ Efficiency EDR includes improvements like a better building envelope and more efficient equipment						
² Total EDR includes efficiency and demand response measures such as photovoltaic (PV) system and batteries						
³ Building complies when source energy, efficiency and total compliance margins are greater than or equal to zero and unmet load hour limits are not exceeded						
• Standard Design PV Capacity: 1.87 kWdc						

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(Page 3 of 14)
Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Mediterranean_1Bedroom.ribd22

ENERGY USE SUMMARY						
Energy Use	Standard Design Source Energy (EDR1) (kBtu/ft ² -yr)	Standard Design TDV Energy (EDR2) (kTDV/ft ² -yr)	Proposed Design Source Energy (EDR1) (kBtu/ft ² -yr)	Proposed Design TDV Energy (EDR2) (kTDV/ft ² -yr)	Compliance Margin (EDR1)	Compliance Margin (EDR2)
Space Heating	7.64	33.56	3.22	24.71	4.42	8.85
Space Cooling	0.8	25.94	0.76	28.05	0.04	-2.11
IAQ Ventilation	0.76	8.12	1.05	11.27	-0.29	-3.15
Water Heating	2.8	29.35	2.04	21.49	0.76	7.86
Self Utilization/Flexibility Credit				0		0
North Facing Efficiency Compliance Total	12	96.97	7.07	85.52	4.93	11.45
Space Heating	7.64	33.56	3.18	24.1	4.46	9.46
Space Cooling	0.8	25.94	0.86	29.65	-0.06	-3.71
IAQ Ventilation	0.76	8.12	1.05	11.27	-0.29	-3.15
Water Heating	2.8	29.35	2.04	21.49	0.76	7.86
Self Utilization/Flexibility Credit				0		0
East Facing Efficiency Compliance Total	12	96.97	7.13	86.51	4.87	10.46

Registration Number:

223-P010038749A-000-000-0000000-0000

Registration Date/Time:

2023-03-31 08:34:41

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CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD
CF1R-PRF-01E
Project Name: Danville ADU Mediterranean 1Bedroom

Calculation Date/Time: 2023-03-30T18:16:58-07:00

(Page 4 of 14)
Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Mediterranean_1Bedroom.ribd22

ENERGY USE SUMMARY						
Energy Use	Standard Design Source Energy (EDR1) (kBtu/ft ² -yr)	Standard Design TDV Energy (EDR2) (kTDV/ft ² -yr)	Proposed Design Source Energy (EDR1) (kBtu/ft ² -yr)	Proposed Design TDV Energy (EDR2) (kTDV/ft ² -yr)	Compliance Margin (EDR1)	Compliance Margin (EDR2)
Space Heating	7.64	33.56	3.05	23.03	4.59	10.53
Space Cooling	0.8	25.94	0.74	27.24	0.06	-1.3
IAQ Ventilation	0.76	8.12	1.05	11.27	-0.29	-3.15
Water Heating	2.8	29.35	2.04	21.49	0.76	7.86
Self Utilization/Flexibility Credit				0		0
South Facing Efficiency Compliance Total	12	96.97	6.88	83.03	5.12	13.94
Space Heating	7.64	33.56	3.11	23.83	4.53	9.73
Space Cooling	0.8	25.94	0.99	34.11	-0.19	-8.17
IAQ Ventilation	0.76	8.12	1.05	11.27	-0.29	-3.15
Water Heating	2.8	29.35	2.04	21.49	0.76	7.86
Self Utilization/Flexibility Credit				0		0
West Facing Efficiency Compliance Total	12	96.97	7.19	90.7	4.81	6.27

Registration Number:

223-P010038749A-000-000-0000000-0000

Registration Date/Time:

2023-03-31 08:34:41

HERS Provider:

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CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD
CF1R-PRF-01E
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Calculation Date/Time: 2023-03-30T18:16:58-07:00

(Page 5 of 14)
Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Mediterranean_1Bedroom.ribd22

ENERGY USE INTENSITY				
	Standard Design (kBtu/ft ² - yr)	Proposed Design (kBtu/ft ² - yr)	Compliance Margin (kBtu/ft ² - yr)	Margin Percentage
North Facing				
Gross EUI¹	27.43	21.1	6.33	23.08
Net EUI²	15.58	9.07	6.51	41.78
East Facing				
Gross EUI¹	27.43	21.21	6.22	22.68
Net EUI²	15.58	9.18	6.4	41.08
South Facing				
Gross EUI¹	27.43	20.87	6.56	23.92
Net EUI²	15.58	8.84	6.74	43.26
West Facing				
Gross EUI¹	27.43	21.3	6.13	22.35
Net EUI²	15.58	9.27	6.31	40.5
Notes 1. Gross EUI is Energy Use Total (not including PV) / Total Building Area. 2. Net EUI is Energy Use Total (including PV) / Total Building Area.				

Registration Number:

223-P010038749A-000-000-0000000-0000

Registration Date/Time:

2023-03-31 08:34:41

HERS Provider:

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CA Building Energy Efficiency Standards - 2022 Residential Compliance

Report Version: 2022.0.000

Schema Version: rev 20220901

Report Generated: 2023-03-30 18:17:46

CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD**CF1R-PRF-01E****Project Name:** Danville ADU Mediterranean 1Bedroom**Calculation Date/Time:** 2023-03-30T18:16:58-07:00**(Page 6 of 14)****Calculation Description:** Title 24 Analysis**Input File Name:** Danville ADU_Mediterranean_1Bedroom.ribd22

REQUIRED PV SYSTEMS											
01	02	03	04	05	06	07	08	09	10	11	12
DC System Size (kWdc)	Exception	Module Type	Array Type	Power Electronics	CFI	Azimuth (deg)	Tilt Input	Array Angle (deg)	Tilt: (x in 12)	Inverter Eff. (%)	Annual Solar Access (%)
2	NA	Standard (14-17%)	Fixed	none	true	105-300	n/a	n/a	<=7:12	96	100

REQUIRED SPECIAL FEATURES											
The following are features that must be installed as condition for meeting the modeled energy performance for this computer analysis.											
- PV System: 2 kWdc - Indoor air quality, balanced fan - IAQ Ventilation System Heat Recovery: minimum 67 SRE and 73 ASRE - IAQ Ventilation System: supply outside air inlet, filter, and H/ERV cores accessible per RACM Reference Manual - IAQ Ventilation System: fault indicator display - Insulation below roof deck - Window overhangs and/or fins - Northwest Energy Efficiency Alliance (NEEA) rated heat pump water heater; specific brand/model, or equivalent, must be installed											

HERS FEATURE SUMMARY											
The following is a summary of the features that must be field-verified by a certified HERS Rater as a condition for meeting the modeled energy performance for this computer analysis. Additional detail is provided in the building tables below. Registered CF2Rs and CF3Rs are required to be completed in the HERS Registry											
- Indoor air quality ventilation - Kitchen range hood - Minimum Airflow - Verified SEER/SEER2 - Verified Refrigerant Charge - Fan Efficacy Watts/CFM - Verified HSPF - Verified heat pump rated heating capacity - Duct leakage testing											

Registration Number:

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(Page 7 of 14)
Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Mediterranean_1Bedroom.ribd22

BUILDING - FEATURES INFORMATION						
01	02	03	04	05	06	07
Project Name	Conditioned Floor Area (ft ²)	Number of Dwelling Units	Number of Bedrooms	Number of Zones	Number of Ventilation Cooling Systems	Number of Water Heating Systems
Danville ADU Mediterranean 1Bedroom	840	1	1	1	0	1

ZONE INFORMATION						
01	02	03	04	05	06	07
Zone Name	Zone Type	HVAC System Name	Zone Floor Area (ft ²)	Avg. Ceiling Height	Water Heating System 1	Status
Accessory Unit	Conditioned	HVAC System 1	840	10	DHW System 1	New

OPAQUE SURFACES							
01	02	03	04	05	06	07	08
Name	Zone	Construction	Azimuth	Orientation	Gross Area (ft ²)	Window and Door Area (ft ²)	Tilt (deg)
Front Wall	Accessory Unit	Exterior 2x6 Wall R-21	0	Front	350	98	90
Left Wall	Accessory Unit	Exterior 2x6 Wall R-21	90	Left	240	15	90
Back Wall	Accessory Unit	Exterior 2x6 Wall R-21	180	Back	350	22	90
Right Wall	Accessory Unit	Exterior 2x6 Wall R-21	270	Right	240	16	90
Ceiling Below Attic	Accessory Unit	R-38 Attic Ceiling	n/a	n/a	840	n/a	n/a

ATTIC							
01	02	03	04	05	06	07	08
Name	Construction	Type	Roof Rise (x in 12)	Roof Reflectance	Roof Emittance	Radiant Barrier	Cool Roof
Attic	Attic Roof	Ventilated	4	0.1	0.85	No	No

Registration Number:

223-P010038749A-000-000-00000000-0000

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2023-03-31 08:34:41

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Calculation Date/Time: 2023-03-30T18:16:58-07:00

(Page 8 of 14)
Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Mediterranean_1Bedroom.ribd22

FENESTRATION / GLAZING													
01	02	03	04	05	06	07	08	09	10	11	12	13	14
Name	Type	Surface	Orientation	Azimuth	Width (ft)	Height (ft)	Mult.	Area (ft ²)	U-factor	U-factor Source	SHGC	SHGC Source	Exterior Shading
FamilyRm 5050	Window	Front Wall	Front	0	5	5	1	25	0.32	NFRC	0.25	NFRC	Bug Screen
Entry 1680 Sidelets	Window	Front Wall	Front	0	1.5	8	2	24	0.32	NFRC	0.25	NFRC	Bug Screen
Front Door	Window	Front Wall	Front	0	3	8	1	24	0.34	NFRC	0.2	NFRC	Bug Screen
Bedroom 5050	Window	Front Wall	Front	0	5	5	1	25	0.32	NFRC	0.25	NFRC	Bug Screen
Kitchen 2630	Window	Left Wall	Left	90	2.5	3	1	7.5	0.32	NFRC	0.25	NFRC	Bug Screen
FamilyRm 2630	Window	Left Wall	Left	90	2.5	3	1	7.5	0.32	NFRC	0.25	NFRC	Bug Screen
Bathroom Back 2020	Window	Back Wall	Back	180	2	2	1	4	0.32	NFRC	0.25	NFRC	Bug Screen
DiningRm 6030	Window	Back Wall	Back	180	6	3	1	18	0.32	NFRC	0.25	NFRC	Bug Screen
Bedroom 4030	Window	Right Wall	Right	270	4	3	1	12	0.32	NFRC	0.25	NFRC	Bug Screen
Bathroom Right 2020	Window	Right Wall	Right	270	2	2	1	4	0.32	NFRC	0.25	NFRC	Bug Screen

OVERHANGS AND FINIS													
01	02	03	04	05	06	07	08	09	10	11	12	13	14
Window	Overhang					Left Fin				Right Fin			
	Depth	Dist Up	Left Extent	Right Extent	Flap Ht.	Depth	Top Up	Dist L	Bot Up	Depth	Top Up	Dist R	Bot Up
FamilyRm 5050	5	0	2	2	0	0	0	0	0	0	0	0	0
Entry 1680 Sidelets	7	0	2	2	0	0	0	0	0	0	0	0	0

Registration Number:

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Registration Date/Time:

2023-03-31 08:34:41

HERS Provider:

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Report Generated: 2023-03-30 18:17:46

CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD
CF1R-PRF-01E
Project Name: Danville ADU Mediterranean 1Bedroom

Calculation Date/Time: 2023-03-30T18:16:58-07:00

(Page 9 of 14)
Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Mediterranean_1Bedroom.ribd22

OVERHANGS AND FINIS													
01	02	03	04	05	06	07	08	09	10	11	12	13	14
Window	Overhang					Left Fin				Right Fin			
	Depth	Dist Up	Left Extent	Right Extent	Flap Ht.	Depth	Top Up	Dist L	Bot Up	Depth	Top Up	Dist R	Bot Up
Front Door	7	0	2	2	0	0	0	0	0	0	0	0	0
Bedroom 5050	1	0	2	2	0	0	0	0	0	0	0	0	0
Kitchen 2630	1	0	2	2	0	0	0	0	0	0	0	0	0
FamilyRm 2630	1	0	2	2	0	0	0	0	0	0	0	0	0
Bathroom Back 2020	1	0	2	2	0	0	0	0	0	0	0	0	0
DiningRm 6030	1	0	2	2	0	0	0	0	0	0	0	0	0
Bedroom 4030	1	0	2	2	0	0	0	0	0	0	0	0	0
Bathroom Right 2020	1	0	2	2	0	0	0	0	0	0	0	0	0

SLAB FLOORS							
01	02	03	04	05	06	07	08
Name	Zone	Area (ft ²)	Perimeter (ft)	Edge Insul. R-value and Depth	Edge Insul. R-value and Depth	Carpeted Fraction	Heated
Slab On Grade	Accessory Unit	840	118	none	0	80%	No

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CF1R-PRF-01E
Project Name: Danville ADU Mediterranean 1Bedroom

Calculation Date/Time: 2023-03-30T18:16:58-07:00

(Page 10 of 14)
Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Mediterranean_1Bedroom.ribd22

OPAQUE SURFACE CONSTRUCTIONS							
01	02	03	04	05	06	07	08
Construction Name	Surface Type	Construction Type	Framing	Total Cavity R-value	Interior / Exterior Continuous R-value	U-factor	Assembly Layers
Exterior 2x6 Wall R-21	Exterior Walls	Wood Framed Wall	2x6 @ 16 in. O. C.	R-21	None / None	0.065	Inside Finish: Gypsum Board Cavity / Frame: R-21 / 2x6 Sheathing / Insulation: Wood Siding/sheathing/decking Exterior Finish: 3 Coat Stucco
Attic Roof	Attic Roofs	Wood Framed Ceiling	2x4 Top Chord of Roof Truss @ 24 in. O. C.	R-13	None / None	0.072	Roofing: 10 PSF (RoofTileAirGap) Tile Gap: present Roof Deck: Wood Siding/sheathing/decking Cavity / Frame: R-13.0 / 2x4 Top Chrd Around Roof Joists: R-0.0 insul.
R-38 Attic Ceiling	Ceilings (below attic)	Wood Framed Ceiling	2x4 Bottom Chord of Truss @ 24 in. O. C.	R-38	None / None	0.025	Over Ceiling Joists: R-28.9 insul. Cavity / Frame: R-9.1 / 2x4 Btm Chrd Inside Finish: Gypsum Board

BUILDING ENVELOPE - HERS VERIFICATION				
01	02	03	04	05
Quality Insulation Installation (QII)	High R-value Spray Foam Insulation	Building Envelope Air Leakage	CFM50	CFM50
Not Required	Not Required	N/A	n/a	n/a

WATER HEATING SYSTEMS								
01	02	03	04	05	06	07	08	09
Name	System Type	Distribution Type	Water Heater Name	Number of Units	Solar Heating System	Compact Distribution	HERS Verification	Water Heater Name (#)
DHW System 1	Domestic Hot Water (DHW)	Standard	Heat Pump WH	1	n/a	None	n/a	Heat Pump WH (1)

Registration Number:

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HERS Provider:

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CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD
CF1R-PRF-01E
Project Name: Danville ADU Mediterranean 1Bedroom

Calculation Date/Time: 2023-03-30T18:16:58-07:00

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Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Mediterranean_1Bedroom.ribd22

WATER HEATERS - NEEA HEAT PUMP							
01	02	03	04	05	06	07	08
Name	# of Units	Tank Vol. (gal)	NEEA Heat Pump Brand	NEEA Heat Pump Model	Tank Location	Duct Inlet Air Source	Duct Outlet Air Source
Heat Pump WH	1	40	Rheem	RheemPROPH40T2R H37515	Outside	Outside	Outside

WATER HEATING - HERS VERIFICATION						
01	02	03	04	05	06	07
Name	Pipe Insulation	Parallel Piping	Compact Distribution	Compact Distribution Type	Recirculation Control	Shower Drain Water Heat Recovery
DHW System 1 - 1/1	Not Required	Not Required	Not Required	None	Not Required	Not Required

SPACE CONDITIONING SYSTEMS								
01	02	03	04	05	06	07	08	09
Name	System Type	Heating Unit Name	Heating Equipment Count	Cooling Unit Name	Cooling Equipment Count	Fan Name	Distribution Name	Required Thermostat Type
HVAC System 1	Heat pump heating cooling	PEAD-A24AA7	1	PEAD-A24AA7	1	HVAC Fan System 1	Distribution System 1	Setback

HVAC - HEAT PUMPS												
01	02	03	04	05	06	07	08	09	10	11	12	13
Name	System Type	Number of Units	Heating				Cooling			Zonally Controlled	Compressor Type	HERS Verification
			Efficiency Type	HSPF / HSPF2 / COP	Cap 47	Cap 17	Efficiency Type	SEER / SEER2	EER / EER / CEER			
PEAD-A24AA7	Central split HP	1	HSPF	10.8	26000	14800	EERSEER	19.6	11.7	Not Zonal	Single Speed	PEAD-A24AA7-hers-htpump

Registration Number:

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2023-03-31 08:34:41

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CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD
CF1R-PRF-01E
Project Name: Danville ADU Mediterranean 1Bedroom

Calculation Date/Time: 2023-03-30T18:16:58-07:00

(Page 12 of 14)
Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Mediterranean_1Bedroom.ribd22

HVAC HEAT PUMPS - HERS VERIFICATION								
01	02	03	04	05	06	07	08	09
Name	Verified Airflow	Airflow Target	Verified EER/EER2	Verified SEER/SEER2	Verified Refrigerant Charge	Verified HSPF/HSPF2	Verified Heating Cap 47	Verified Heating Cap 17
PEAD-A24AA7-hers-htpump	Required	350	Not Required	Required	Yes	Yes	Yes	Yes

HVAC - DISTRIBUTION SYSTEMS											
01	02	03	04	05	06	07	08	09	10	11	12
Name	Type	Design Type	Duct Ins. R-value		Duct Location		Surface Area		Bypass Duct	Duct Leakage	HERS Verification
			Supply	Return	Supply	Return	Supply	Return			
Distribution System 1	Unconditioned attic	Non-Verified	R-6	R-6	Attic	Attic	n/a	n/a	No Bypass Duct	Sealed and Tested	Distribution System 1-hers-dist

HVAC DISTRIBUTION - HERS VERIFICATION								
01	02	03	04	05	06	07	08	09
Name	Duct Leakage Verification	Duct Leakage Target (%)	Verified Duct Location	Verified Duct Design	Buried Ducts	Deeply Buried Ducts	Low-leakage Air Handler	Low Leakage Ducts Entirely in Conditioned Space
Distribution System 1-hers-dist	Yes	5.0	Not Required	Not Required	Not Required	Credit not taken	Not Required	No

HVAC - FAN SYSTEMS			
01	02	03	04
Name	Type	Fan Power (Watts/CFM)	Name
HVAC Fan System 1	HVAC Fan	0.45	HVAC Fan System 1-hers-fan

Registration Number:

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Registration Date/Time:

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HERS Provider:

CalCERTS inc.

CA Building Energy Efficiency Standards - 2022 Residential Compliance

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CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD**CF1R-PRF-01E****Project Name:** Danville ADU Mediterranean 1Bedroom**Calculation Date/Time:** 2023-03-30T18:16:58-07:00**(Page 13 of 14)****Calculation Description:** Title 24 Analysis**Input File Name:** Danville ADU_Mediterranean_1Bedroom.ribd22

HVAC FAN SYSTEMS - HERS VERIFICATION		
01	02	03
Name	Verified Fan Watt Draw	Required Fan Efficacy (Watts/CFM)
HVAC Fan System 1-hers-fan	Required	0.45

INDOOR AIR QUALITY (IAQ) FANS								
01	02	03	04	05	06	07	08	09
Dwelling Unit	Airflow (CFM)	Fan Efficacy (W/CFM)	IAQ Fan Type	Includes Heat/Energy Recovery?	IAQ Recovery Effectiveness - SRE	Includes Fault Indicator Display?	HERS Verification	Status
SFam IAQVentRpt 1-1	49	0.816326	Balanced	Yes	73	No	Yes	



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CA Building Energy Efficiency Standards - 2022 Residential Compliance

Report Version: 2022.0.000

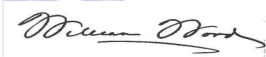
Schema Version: rev 20220901

Report Generated: 2023-03-30 18:17:46

CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD

CF1R-PRF-01E

Project Name: Danville ADU Mediterranean 1Bedroom**Calculation Date/Time:** 2023-03-30T18:16:58-07:00**(Page 14 of 14)****Calculation Description:** Title 24 Analysis**Input File Name:** Danville ADU_Mediterranean_1Bedroom.ribd22

DOCUMENTATION AUTHOR'S DECLARATION STATEMENT			
1. I certify that this Certificate of Compliance documentation is accurate and complete.			
Documentation Author Name: Jason E Bennett		Documentation Author Signature: 	
Company: DP Advanced Engineering		Signature Date: 2023-03-30 19:17:10	
Address: 3381 Walnut Blvd. Ste. 220		CEA/ HERS Certification Identification (If applicable): R19-21-30042	
City/State/Zip: Brentwood, CA 94513		Phone: 925-516-3502	
RESPONSIBLE PERSON'S DECLARATION STATEMENT			
I certify the following under penalty of perjury, under the laws of the State of California: 1. I am eligible under Division 3 of the Business and Professions Code to accept responsibility for the building design identified on this Certificate of Compliance. 2. I certify that the energy features and performance specifications identified on this Certificate of Compliance conform to the requirements of Title 24, Part 1 and Part 6 of the California Code of Regulations. 3. The building design features or system design features identified on this Certificate of Compliance are consistent with the information provided on other applicable compliance documents, worksheets, calculations, plans and specifications submitted to the enforcement agency for approval with this building permit application.			
Responsible Designer Name: William Wood		Responsible Designer Signature: 	
Company: William Wood Architects		Date Signed: 2023-03-31 08:34:41	
Address: 301 Hartz Ave #203		License: C13874	
City/State/Zip: Danville, CA 94526		Phone: 925-820-8233	

Digitally signed by CalCERTS. This digital signature is provided in order to secure the content of this registered document, and in no way implies

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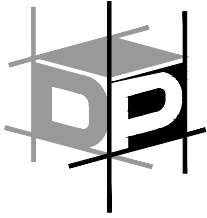
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ENVELOPE INSULATION

EXTERIOR WALLS:

R-21 IN 2x6

FOUNDATION:

SLAB, NOT INSULATED

ROOF OVER LIVING SPACE:

R-38 BETWEEN CEILING JOISTS

R-13 BETWEEN RAFTERS

WINDOW PERFORMANCE

MAXIMUM WINDOW PERFORMANCE VALUES (U-FACTOR / SHGC)

WINDOWS: .30/.23

FRENCH DOORS: .32/.16

DOMESTIC HOT WATER

RHEEM 40-GALLON HEAT PUMP WATER HEATER OR EQUIV.

SOLAR (STUDIO)

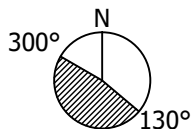
(NOT REQUIRED; CALCULATED SIZE IS <1.8 KW)

SOLAR (1 BEDROOM)

2.0 KW MINIMUM

ACCEPTABLE ORIENTATION:

(DEGREES EAST OF TRUE NORTH)



HERS VERIFICATIONS

- INDOOR AIR QUALITY VENTILATION
- KITCHEN RANGE HOOD
- MINIMUM AIRFLOW [350 CFM/TON]
- VERIFIED SEER [19.6]
- VERIFIED REFRIGERANT CHARGE
- FAN EFFICACY WATTS/CFM [0.45]
- VERIFIED HSPF [10.8]
- VERIFIED HEAT PUMP RATED HEATING CAPACITY
- DUCT LEAKAGE TESTING [5%]

SPACE HEATING & COOLING

HEAT PUMP (10.2 HSPF, 19.6 SEER, 11.7 EER)

MITSUBISHI PEAD-A24AA7 + PUZ-A24NHA7

AHRI # 201754661

DUCTS

SUPPLY AIR DUCTS IN ATTIC

DUCTS IN UNCONDITIONED SPACE TO HAVE R-6 INSULATION

INDOOR AIR QUALITY

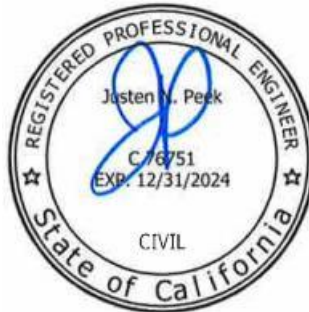
BROAN ERV100S ENERGY RECOVERY VENTILATOR OR EQUIV.

Structural Calculations

PROJECT: Town of Danville 1-Bedroom ADU
ADDRESS: Danville, California

JOB No.: W021420
DELTA: Permit Submittal
DATE: March 24, 2023

CLIENT: Town of Danville



GOVERNING CODE: California Building Code, 2022 Edition
CONSTRUCTION: One-Story Wood Framed Building

STRUCTURE:
Vertical Load System - Wood Framed Roof
Lateral Load System - Wood Framed Shear Walls



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

STRUCTURAL CALCULATIONS ARE BASED ON THE FOLLOWING CRITERIA, UNLESS NOTED OTHERWISE

STRUCTURAL MATERIALS:

Structural Steel:

W Shapes	ASTM A992, Fy = 50 ksi
Shapes (M, S, HP, C, MC, L)	ASTM A572, Grade 50
Pipe	ASTM A53, Grade B
Tube (HSS)	ASTM A500, Grade B
Plates and Bars	ASTM A572, Grade 50
Welding	Current AWS D1.1
Bolts - Unfinished	ASTM A307
Bolts - High Strength (HSB)	ASTM A325 SC/N/X
Threaded Rods	ASTM A449
Light Gage Steel Studs and Joists	ASTM A653, Grade 50
Welding (Light Gage)	Current AWS D1.3

Concrete:

Slab on Grade	2500 psi @ 28 Days
Foundation	2500 psi @ 28 Days
Tilt-Up Wall Panels	4000 psi @ 28 Days
Structural Concrete	3000 psi @ 28 Days
Reinforcing Steel (#3 Rebar)	ASTM A615, Grade 40
Reinforcing Steel (#4 & larger)	ASTM A615, Grade 60
Welding (Reinf.).....	Current AWS D1.4

Wood:

2" to 4" Thick x 2" and Wider	DF No. 2 or better
Joists and Planks	DF No. 2 or better
Beams and Stringers	DF No. 1 or better
Posts and Timbers	DF No. 1 or better
Glu-lam Beams (simple span)	2400F-V4 DF/DF
Glu-lam Beams (cantilevers)	2400F-V8 DF/DF
Sheathing	Exposure 1, Grade C-D, C-C



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

STRUCTURAL CALCULATIONS ARE BASED ON THE FOLLOWING CRITERIA, UNLESS NOTED OTHERWISE

LOADING SCHEDULE:

Roof Dead Load:

Max. Roof Pitch /12:

5

Roofing:	Concrete Tile	10.8 psf
Sheathing:	1/2" Plywood	1.8 psf
Insulation:	R38 Insulation Ceiling & R19 Insulation Roof Deck	1.0 psf
Framing:	Trusses @ 24" o.c.	2.5 psf
Ceiling:	5/8" Gyp. Installed on Bottom of Roof Framing	2.8 psf
Sprinklers:	Automatic Fire Sprinklers	1.0 psf
Misc:	HVAC + Miscellaneous	2.1 psf

22.0 psf

Photovoltaic Dead Load:

Non-concurrent with Live Load

3.0 psf

Roof Live Load:

Sloped Roof (Reducible)

20.0 psf

Exterior Wall Load:

Wall Type: 2x6 @ 16 in, 5/8" Gyp, Insulated, 7/8" Stucco

17.0 psf

Interior Wall Load:

Wall Type: 2x4 @ 16 in, (2) 5/8" Gyp, Insulated

7.0 psf



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

SEISMIC LOADS: ASCE 7-16 SECTION 12.8 (EQUIV. LATERAL FORCE):

SITE INFORMATION:

S_s 2.500 ASCE 7-16 Sect. 11.4.2
 S_1 1.200 ASCE 7-16 Sect. 11.4.2
 Site Class..... D Per Geotech Report
 Risk Category II ASCE 7-16 Table 1.5-1
 Importance Factor 1.0 ASCE 7-16 Table 1.5-2

Site coefficients and adjusted maximum considered eq. spectral response accel. parameters

F_a 1.200 ASCE 7-16 Sect. 11.4.4
 F_v 1.700 ASCE 7-16 Sect. 11.4.4

Table 11.4-1 Short-Period Site Coefficient F_a

Site Class	$S_s \leq 0.25$	$S_s = 0.50$	$S_s = 0.75$	$S_s = 1.00$	$S_s = 1.25$	$S_s \geq 1.50$
A	0.8	0.8	0.8	0.8	0.8	0.8
B	0.9	0.9	0.9	0.9	0.9	0.9
C	1.3	1.3	1.2	1.2	1.2	1.2
D	1.6	1.4	1.2	1.1	1.0	1.0
E	2.4	1.7	1.3	1.2	1.2	1.2
F	Site-Specific Response Analysis Required					

Table 11.4-2 Short-Period Site Coefficient F_v

Site Class	$S_1 \leq 0.10$	$S_1 = 0.20$	$S_1 = 0.30$	$S_1 = 0.40$	$S_1 = 0.50$	$S_1 \geq 0.60$
A	0.8	0.8	0.8	0.8	0.8	0.8
B	0.8	0.8	0.8	0.8	0.8	0.8
C	1.5	1.5	1.5	1.5	1.5	1.4
D	2.4	2.2	2	1.9	1.8	1.7
E	4.2	3.3	2.8	2.4	2.2	2
F	Site-Specific Response Analysis Required					

$S_{MS} = F_a S_s$ 3.000 (EQ. 11.4-1)
 $S_{M1} = F_v S_1$ 2.040 (EQ. 11.4-2)
 $S_{DS} = (2/3) S_{MS}$ 2.000 g (EQ. 11.4-3)
 $S_{D1} = (2/3) S_{M1}$ 1.360 g (EQ. 11.4-4)

Seismic Design Category Based on Short-Period Response Accelerations:

VALUE OF S_{DS}	1 OR 11	111	1V	
$S_{DS} < 0.167g$	A	A	A	S1 IS GREATER THAN 0.75 USE CATEGORY E PER CBC 1613.2.5
$0.167g \leq S_{DS} < 0.33g$	B	B	C	
$0.33g \leq S_{DS} < 0.50g$	C	C	D	
$0.50g \leq S_{DS}$	D	D	D	

Seismic Design Category Based on 1-Second Response Accelerations:

VALUE OF S_{D1}	1 OR 11	111	1V	
$S_{D1} < 0.067g$	A	A	A	S1 IS GREATER THAN 0.75 USE CATEGORY E PER CBC 1613.2.5
$0.067g \leq S_{D1} < 0.133g$	B	B	C	
$0.133g \leq S_{D1} < 0.20g$	C	C	D	
$0.20g \leq S_{D1}$	D	D	D	



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

SEISMIC LOADS: ASCE 7-16 SECTION 12.8 (CONT.):

BUILDING INFORMATION:

Building Height, h_n 15.5 ft
 Mean Roof Height, H_m 12.8 ft
 Eave Height, h 10.0 ft

Building Depth:

Bldg Depth (Roof Level)..... 30.0 ft

Building Width:

Bldg Width (Roof Level)..... 35.0 ft

EQUIVALENT LATERAL FORCE PROCEDURE:

Seismic Base Shear:

R (ASCE 7-16 Table 12.2-1)..... 6.5
 C_t 0.020
 X 0.75
 $T_a = C_t(h_n)^X$ 0.16 sec

Seismic Response Coefficient:

$C_S = S_{DS}/(R/I)$ 0.308 (EQ. 12.8-2)
 $C_S = S_{D1}/T(R/I)$ 1.339 (EQ. 12.8-3)
 $C_S = 0.044S_{DS}I$ 0.088 (EQ. 12.8-5)
 $C_S = 0.5S_1/(R/I)$ 0.092 (EQ. 12.8-6)

Seismic Base Shear:

$V = C_S W$ **0.308 W** (EQ. 12.8-1)

BUILDING WEIGHTS:

SEISMIC BASE SHEAR:

Roof Loads:	AREA (sq ft)	WEIGHT (psf)	TOTAL (lb)
Roof Area	1082	22.0	23831
Roof Top Deck	0	15.0	0
Photovoltaic Area	1082	3.0	3246

Building DL = 40254 lb
 Seismic Coef. = 0.308
 $k = 1.00$
Base Shear = 12386 lb

Roof Loads:	LENGTH (ft)	WEIGHT (psf)	TOTAL (lb)
Linear Feet of Ext. Wall	118	17.0	8597
Linear Feet of Int. Wall	130	7.0	4581

*Note: Approximate Fenestration Percentage: 15 %
TOTAL ROOF LOAD (lb): 40254

CALCULATE SEISMIC SHEAR LOADS:

VERTICAL DISTRIBUTION OF SEISMIC FORCES					
Level	w_x (lb)	Top Plate Height (ft)	h_x (ft)	F_x (lb)	Unit Shear (psf)
ROOF	40254	10.1	12.8	12386	11.4
			$\sum w_i * h_i^k =$	513244	

REDUNDANCY FACTOR (ASCE 7-16 SECTION 12.3.4):

Seismic Design Cat: **USE CATEGORY E**
 Roof East/ West: **1.0**

*Note: Refer to calculations on following sheets
 Roof North/South: **1.3**

SUMMARY OF SEISMIC DESIGN LOADS:

Level	Direction	SEISMIC	UNIT	Redundancy	ASD DESIGN	ASD DESIGN
		FORCE (lb)	SHEAR (psf)	Factor	FORCE (lb)	LOAD (psf)
Roof	North/South	12386	11.4	1.3	11271	10.4
Roof	East/West	12386	11.4	1.0	8670	8.0



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

WIND ANALYSIS (ENCLOSED, PARTIALLY ENCLOSED AND OPEN BUILDINGS OF ALL HEIGHTS)

MAIN WIND-FORCE RESISTING SYSTEM:

Design Wind Speed..... 95 mph
 Wind Exposure..... C
 Site Elevation..... Unknown Feet
 Topographic factor, ASCE 7-10 Section 26.8.2 -
 K_{zt} (Assumed Flat Area)..... 1.00

BUILDING INFORMATION:

Building Height, h_n 15.5 ft
 Mean Roof Height, H_m 12.8 ft
 Eave Height, h 10.0 ft

Building Depth "B":

Bldg Depth (Roof Level)..... 30.0 ft
 Bldg Depth (Floor Level)..... 0.0 ft

Building Width "L":

Bldg Width (Roof Level)..... 35.0 ft
 Bldg Width (Floor Level)..... 0.0 ft

DETERMINE WIND LOAD PARAMETERS:

DIRECTIONALITY FACTOR (K_d): 0.85 ** See ASCE 7-16 Section 26.6 and Table 26.6-1
 TOPOGRAPHIC FACTOR (K_{zt}): 1.00 ** See ASCE 7-16 Section 26.8 and Figure 26.8-1
 GUST FACTOR (G): 0.85 ** See ASCE 7-16 Section 26.11.1
 ENCLOSURE CLASSIFICATION Enclosed ** See ASCE 7-16 Section 26.12
 INTERNAL PRESSURE (C_{pI}) 0.18 ** See ASCE 7-16 Section 26.12 and Table 26.13-1
 GROUND ELEVATION FACTOR (K_e) 1.00 ** See ASCE 7-16 Table 26.9-1, Note 2

Table 26.10-1 Velocity Pressure Coefficients, K_z

Height Above Ground (ft)	Exposure Category		
	B	C	D
15.0	0.57	0.85	1.03
20.0	0.62	0.90	1.08
25.0	0.66	0.94	1.12
30.0	0.70	0.98	1.16
35.0	0.73	1.01	1.19
40.0	0.76	1.04	1.22
45.0	0.79	1.07	1.25
50.0	0.81	1.09	1.27
55.0	0.83	1.11	1.29
60.0	0.85	1.13	1.31

VELOCITY COEFFICIENT (K_z): 0.85
 ** See ASCE 7-16 Table 26.10.1

DETERMINE VELOCITY PRESSURE:

$q_z = 0.00256 K_z K_{zt} K_d K_e V^2$: 16.69 psf



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

WIND ANALYSIS (ENCLOSED, PARTIALLY ENCLOSED AND OPEN BUILDINGS OF ALL HEIGHTS)

DETERMINE WALL PRESSURE:

Wall Pressure Coefficients, ASCE-7 (Figure 27..3-1)

Surface	L/B	Cp	Use With
Windward Wall	All Values	0.8	q _z
Leeward Wall	0-1	-0.5	q _h
	2	-0.3	
	>4	-0.2	
Side Wall	All Values	-0.7	q _h

Determine Cp Values

Level	Direction	L/B Values	Windward C _p Value	Leeward C _p Value
Roof	East/West	1.17	0.80	-0.467

Level	Direction	L/B Values	Windward C _p Value	Leeward C _p Value
Roof	North/South	0.86	0.80	-0.500

Windward Pressures:

Plan North/South

CASE 1: 14.36
CASE 2: 8.35

Plan East/West

CASE 1: 14.36
CASE 2: 8.35

Leeward Pressures:

Plan North/South

CASE 1: 4.09
CASE 2: 10.10

Plan East/West

CASE 1: 3.62
CASE 2: 9.63

SUMMARY OF WIND FORCE DESIGN LOADS

Level	Direction	WIND DESIGN	UNIT		ASD DESIGN	ASD DESIGN
		FORCE (lb)	SHEAR (plf)		FORCE (lb)	LOAD (plf)
Roof	North/South	4976	142		2986	85
Roof	East/West	4156	139		2494	83



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

SHEARWALL DESIGN: ROOF LEVEL

Shearwall Framing: 3/8" CDX ply with 8d Nailing

SHEAR LINE ID:	1	2
-----------------------	----------	----------

DETERMINATION OF LATERAL DESIGN LOADS:

Grid Line Trib. Area (sq ft):	481	601
Seismic Shear Load (psf):	8.0	8.0
Seismic Shear Force (lb):	3854	4816

Grid Line Trib. Width (ft):	12	18
Wind Design Load (plf):	83	83
Wind Design Force (lb):	997	1496

SHEARWALL LENGTHS:

Wall Length 1 (ft):	8.0	5.0
Wall Length 2 (ft):	8.0	5.0
Wall Length 3 (ft):	0.0	4.0
Wall Length 4 (ft):	0.0	0.0
Wall Length 5 (ft):	0.0	0.0

SHEARWALL RIGIDITY DESIGN:

Wall 1 Rigidity K_i (kip/in):	5.42	3.54
Wall 2 Rigidity K_i (kip/in):	5.42	3.54
Wall 3 Rigidity K_i (kip/in):		2.51
Wall 4 Rigidity K_i (kip/in):		
Wall 5 Rigidity K_i (kip/in):		

NAILING DESIGN BASED ON SDPWS SECTION 4.3.4 CAPACITY ADJ:

Wall 1 Design Shear (plf):	241	356
<i>Sec. 4.3.4.2 Capacity Reduction</i>	1.00	1.00
Wall 2 Design Shear (plf):	241	356
<i>Sec. 4.3.4.2 Capacity Reduction</i>	1.00	1.00
Wall 3 Design Shear (plf):		336
<i>Sec. 4.3.4.2 Capacity Reduction</i>		0.93
Wall 4 Design Shear (plf):		
<i>Sec. 4.3.4.2 Capacity Reduction</i>		
Wall 5 Design Shear (plf):		
<i>Sec. 4.3.4.2 Capacity Reduction</i>		

Shear Capacity (plf):	260	490
-----------------------	-----	-----

Shear Designation #: **Type 1** **Type 3**

REDUNDANCY FACTOR ANALYSIS (ASCE 7-16 SECTION 12.3.4):

Wall 1 Capacity (%):	0.19	0.22
Wall 2 Capacity (%):	0.19	0.22
Wall 3 Capacity (%):		0.18
Wall 4 Capacity (%):		
Wall 5 Capacity (%):		

Redundancy Factor: **Use 1.0** **Use 1.0**



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

SHEARWALL DESIGN (CONT.): ROOF LEVEL

SHEAR LINE ID:	1	2
-----------------------	----------	----------

SHEARWALL OVERTURNING DESIGN:

Wall Height (ft):	10.1	10.1
Wall Weight (psf):	17.0	17.0
Roof Trib. Width (ft):	2	2
Roof Weight (psf):	22.0	22.0

SEISMIC UPLIFT FORCE (lb): Load Combo (ASCE 7-16): (0.6-0.14*SDS)D+0.7E

Wall Dead Load (plf):	69	69
Wall 1 Uplift (lb):	2153	3415
Wall 2 Uplift (lb):	2153	3415
Wall 3 Uplift (lb):		3034
Wall 4 Uplift (lb):		
Wall 5 Uplift (lb):		

WIND UPLIFT FORCE (lb): Load Combo (ASCE 7-16): 0.6D+0.6W

Wall Dead Load (plf):	129	129
Wall 1 Uplift (lb):	111	791
Wall 2 Uplift (lb):	111	791
Wall 3 Uplift (lb):		727
Wall 4 Uplift (lb):		
Wall 5 Uplift (lb):		

HOLDOWN TYPE:

Wall 1 Holdown Type:	HDU2	HDU4
Wall 2 Holdown Type:	HDU2	HDU4
Wall 3 Holdown Type:		HDU4
Wall 4 Holdown Type:		
Wall 5 Holdown Type:		

STORY DRIFT CHECK ($C_d=4$): Due to approximate nature of rigidity calculation methods, +/- 10% variation OK.

Wall 1 Defl. Check (in):	1.7	2.4
Wall 2 Defl. Check (in):	1.7	2.4
Wall 3 Defl. Check (in):		2.4
Wall 4 Defl. Check (in):		
Wall 5 Defl. Check (in):		
Wall 1 Drift Check (in):	OK	OK
Wall 2 Drift Check (in):	OK	OK
Wall 3 Drift Check (in):		OK
Wall 4 Drift Check (in):		
Wall 5 Drift Check (in):		

AREA VERIFICATION CHECK:

Seismic Trib Area Sum: 1082 sq ft

Trib Width Sum: 30 ft



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

SHEARWALL DESIGN: ROOF LEVEL

Shearwall Framing: 3/8" CDX ply with 8d Nailing

SHEAR LINE ID:	A	B
-----------------------	----------	----------

DETERMINATION OF LATERAL DESIGN LOADS:

Grid Line Trib. Area (sq ft):	567	515
Seismic Shear Load (psf):	10.4	10.4
Seismic Shear Force (lb):	5906	5365

Grid Line Trib. Width (ft):	18	18
Wind Design Load (plf):	85	85
Wind Design Force (lb):	1493	1493

SHEARWALL LENGTHS:

Wall Length 1 (ft):	11.5	9.5
Wall Length 2 (ft):	0.0	0.0
Wall Length 3 (ft):	0.0	0.0
Wall Length 4 (ft):	0.0	0.0
Wall Length 5 (ft):	0.0	0.0

SHEARWALL RIGIDITY DESIGN:

Wall 1 Rigidity K_i (kip/in):	14.02	12.53
Wall 2 Rigidity K_i (kip/in):		
Wall 3 Rigidity K_i (kip/in):		
Wall 4 Rigidity K_i (kip/in):		
Wall 5 Rigidity K_i (kip/in):		

NAILING DESIGN BASED ON SDPWS SECTION 4.3.4 CAPACITY ADJ:

Wall 1 Design Shear (plf):	514	565
<i>Sec. 4.3.4.2 Capacity Reduction</i>	1.00	1.00
Wall 2 Design Shear (plf):		
<i>Sec. 4.3.4.2 Capacity Reduction</i>		
Wall 3 Design Shear (plf):		
<i>Sec. 4.3.4.2 Capacity Reduction</i>		
Wall 4 Design Shear (plf):		
<i>Sec. 4.3.4.2 Capacity Reduction</i>		
Wall 5 Design Shear (plf):		
<i>Sec. 4.3.4.2 Capacity Reduction</i>		

Shear Capacity (plf):	600	600
-----------------------	-----	-----

Shear Designation #: Type 4 Type 4

REDUNDANCY FACTOR ANALYSIS (ASCE 7-16 SECTION 12.3.4):

Wall 1 Capacity (%):	0.45
Wall 2 Capacity (%):	
Wall 3 Capacity (%):	
Wall 4 Capacity (%):	
Wall 5 Capacity (%):	

Redundancy Factor: Use 1.0 Use 1.3



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

SHEARWALL DESIGN (CONT.): ROOF LEVEL

SHEAR LINE ID:	A	B
-----------------------	----------	----------

SHEARWALL OVERTURNING DESIGN:

Wall Height (ft):	10.1	10.1
Wall Weight (psf):	17.0	17.0
Roof Trib. Width (ft):	0	0
Roof Weight (psf):	22.0	22.0

SEISMIC UPLIFT FORCE (lb): Load Combo (ASCE 7-16): (0.6-0.14*SDS)D+0.7E

Wall Dead Load (plf):	55	55
Wall 1 Uplift (lb):	4863	5434
Wall 2 Uplift (lb):		
Wall 3 Uplift (lb):		
Wall 4 Uplift (lb):		
Wall 5 Uplift (lb):		

WIND UPLIFT FORCE (lb): Load Combo (ASCE 7-16): 0.6D+0.6W

Wall Dead Load (plf):	103	103
Wall 1 Uplift (lb):	718	1096
Wall 2 Uplift (lb):		
Wall 3 Uplift (lb):		
Wall 4 Uplift (lb):		
Wall 5 Uplift (lb):		

HOLDOWN TYPE:

Wall 1 Holdown Type:	HDU5	HDU5
Wall 2 Holdown Type:		
Wall 3 Holdown Type:		
Wall 4 Holdown Type:		
Wall 5 Holdown Type:		

STORY DRIFT CHECK ($C_d=4$): Due to approximate nature of rigidity calculation methods, +/- 10% variation OK.

Wall 1 Defl. Check (in):	2.1	2.4
Wall 2 Defl. Check (in):		
Wall 3 Defl. Check (in):		
Wall 4 Defl. Check (in):		
Wall 5 Defl. Check (in):		
Wall 1 Drift Check (in):	OK	OK
Wall 2 Drift Check (in):		
Wall 3 Drift Check (in):		
Wall 4 Drift Check (in):		
Wall 5 Drift Check (in):		

AREA VERIFICATION CHECK:

Seismic Trib Area Sum: 1082 sq ft

Trib Width Sum: 35 ft



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

ROOF DIAPHRAGM DESIGN:

DIMENSIONS:

L = 35 ft

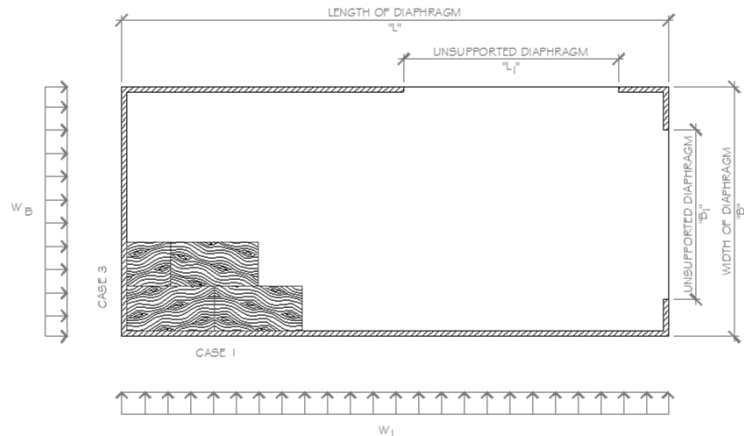
B = 30 ft

B₁ = 0 ft

L₁ = 0 ft

FRAMING MATERIALS:

Plywood Panel Grade **CD, CC**
 Minimum Nominal Framing Size: **2 x**
 Nominal Plywood Thickness: **15/32**
 Common Nail Size: **8 d**
 Wall framing size: **2x4**
 Specific Gravity of Framing Members: **0.5**



LATERAL FORCE ALONG L SIDE:

$W_L = 313 \text{ plf}$

LATERAL FORCE ALONG B SIDE:

$W_B = 280 \text{ plf}$

ANALYSIS -

The diaphragm is considered flexible if its maximum lateral deformation is more than 2x the average shearwall deflection of the associated story.

Diaphragm Ratio: $L/B = 1.17 < 3$ Diaphragm OK

Shear_{MAX} along Side L: $V_L = W_B * B / 2 * L = 120 \text{ plf}$

Shear_{MAX} along Side B: $V_B = W_L * L / 2 * B = 182 \text{ plf}$

Chord Forces:

Side L: $T_L = C_L = W_L * L^2 / 8 * B = 1595 \text{ lb}$

** Min. Blkg Req'd @ 96 in o.c.

Side B: $T_B = C_B = W_B * B^2 / 8 * L = 901 \text{ lb}$

** Min. Blkg Req'd @ 96 in o.c.

Drag Forces:

Side L: $F_L = V_L * L_1 = 0 \text{ lb}$

** Min. Strap Req'd - NONE

Side B: $F_B = V_B * B_1 = 0 \text{ lb}$

** Min. Strap Req'd - NONE

DIAPHRAGM DEFLECTION -

* per APA: Supplement For Shearwall and Diaphragms

$$D = \frac{5V_L^3}{8EAb} + \frac{V_L}{4Gt} + 0.188Le_n + \frac{S(D_cX)}{2b}$$

Chord Properties:

Area (in²): 5.25

G_vt_v of ply (lb/in): 83500

E (psi): 1600000

e_n (in) = (V_n/616)^{3.018}: 0.045

ν (plf): 182

S (D_cX) (in): 2.50

D = 0.40 in

D_{ALLOW} = 0.025 * Story Height = 3.8 in

Deflection OK



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 45009

0

ROOF DIAPHRAGM DESIGN:

NAILING PATTERN -

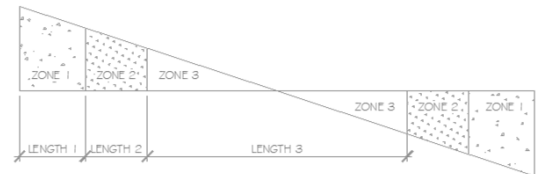
Panel Grade	Common Nail	Min. Pen. (in)	Min. Thick. (in)	Member Width (in)	Blocked Nail Spacing				Unblocked	
					Boundary / Other Edges					
					6 / 6	4 / 6	2.5 / 4	2 / 3	Case 1	Others
CD, CC	8 d	1 1/2	15/32	2	270	360	530	600	240	180

DIAPHRAGM NAILING DESIGN -

Along Length 'L'

Zone 1: 35 FT WIDE x 30 FT DEEP

UNBLOCKED 15/32 SHEATHING WITH 8 d COMMON NAILS
6.0 IN O.C. BOUNDARY/ 12 IN O.C. EDGES/ 12 IN O.C. FIELD

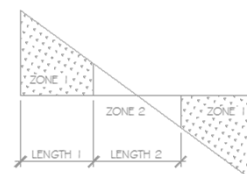
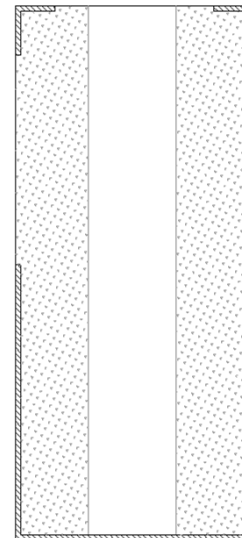


DIAPHRAGM NAILING DESIGN -

Along Length 'B'

Zone 1: 30 FT WIDE x 35 FT DEEP

UNBLOCKED 15/32 SHEATHING WITH 8 d COMMON NAILS
6.0 IN O.C. BOUNDARY/ 12 IN O.C. EDGES/ 12 IN O.C. FIELD



Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

(c) ENERCALC INC 1983-2022

DESCRIPTION: Typical Header

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Load Combination Set : ASCE 7-16

Material Properties

Analysis Method : Allowable Stress Design

Load Combination : ASCE 7-16

Wood Species : Douglas Fir-Larch

Wood Grade : No.1

Beam Bracing : Beam is Fully Braced against lateral-torsional buckling

Fb + 1,350.0 psi

Fb - 1,350.0 psi

Fc - Prll 925.0 psi

Fc - Perp 625.0 psi

Fv 170.0 psi

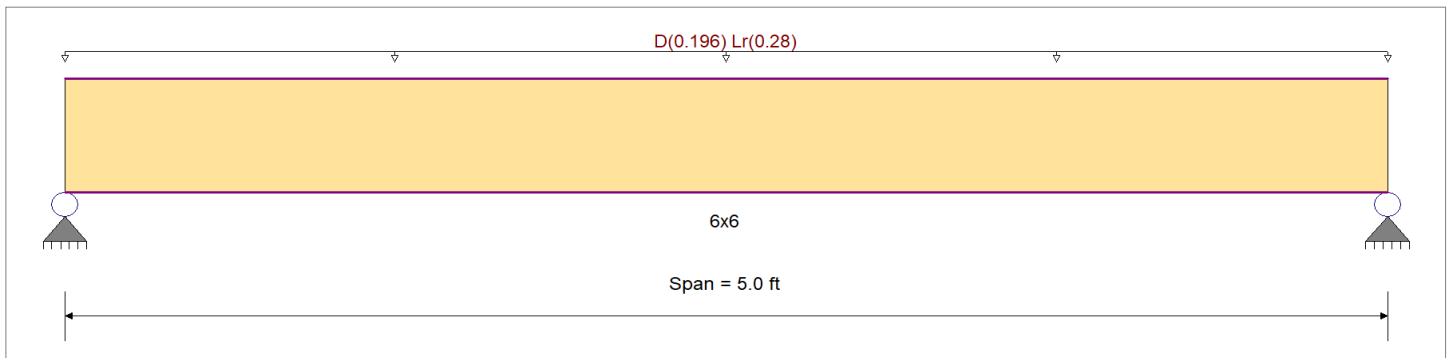
Ft 675.0 psi

E : Modulus of Elasticity

Ebend- xx 1,600.0ksi

Eminbend - xx 580.0ksi

Density 31.210pcf



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading

Uniform Load : D = 0.0140, Lr = 0.020 ksf, Tributary Width = 14.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio				Maximum Shear Stress Ratio			
Section used for this span	=	0.387	1	Section used for this span	=	0.230	1
fb: Actual	=	652.59 psi		fv: Actual	=	48.90 psi	
F'b	=	1,687.50 psi		F'v	=	212.50 psi	
Load Combination		+D+Lr		Load Combination		+D+Lr	
Location of maximum on span	=	2.500 ft		Location of maximum on span	=	0.000 ft	
Span # where maximum occurs	=	Span # 1		Span # where maximum occurs	=	Span # 1	
Maximum Deflection							
Max Downward Transient Deflection	0.032 in	Ratio =	1848 >= 360	Span: 1 : Lr Only			
Max Upward Transient Deflection	0 in	Ratio =	0 < 360	n/a			
Max Downward Total Deflection	0.056 in	Ratio =	1072 >= 240	Span: 1 : +D+Lr			
Max Upward Total Deflection	0 in	Ratio =	0 < 240	n/a			

Maximum Forces & Stresses for Load Combinations

Load Combination		Max Stress Ratios										Moment Values			Shear Values				
		Segment Length	Span #	M	V	CD	CM	C _t	CLx	C _F	C _{fu}	C _i	C _r	M	fb	F'b	V	fv	F'v
D Only																0.0	0.00	0.0	0.0
	Length = 5.0 ft	1	0.225	0.134	0.90	1.00	1.00	1.00	1.000	1.00	1.00	1.00	0.63	273.9	1,215.0	0.41	20.5	153.0	
+D+Lr						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0	
	Length = 5.0 ft	1	0.387	0.230	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	1.51	652.6	1,687.5	0.99	48.9	212.5	
+D+0.750Lr						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0	
	Length = 5.0 ft	1	0.331	0.197	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	1.29	557.9	1,687.5	0.84	41.8	212.5	
+0.60D						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0	
	Length = 5.0 ft	1	0.076	0.045	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	0.38	164.4	2,160.0	0.25	12.3	272.0	

Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

(c) ENERCALC INC 1983-2022

DESCRIPTION: Typical Header

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+D+Lr	1	0.0559	2.518		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1

Values in KIPS

Load Combination	Support 1	Support 2
Max Upward from all Load Conditions	1.206	1.206
Max Upward from Load Combinations	1.206	1.206
Max Upward from Load Cases	0.700	0.700
D Only	0.506	0.506
+D+Lr	1.206	1.206
+D+0.750Lr	1.031	1.031
+0.60D	0.304	0.304
Lr Only	0.700	0.700

Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC#: KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

(c) ENERCALC INC 1983-2022

DESCRIPTION: B1

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Load Combination Set : ASCE 7-16

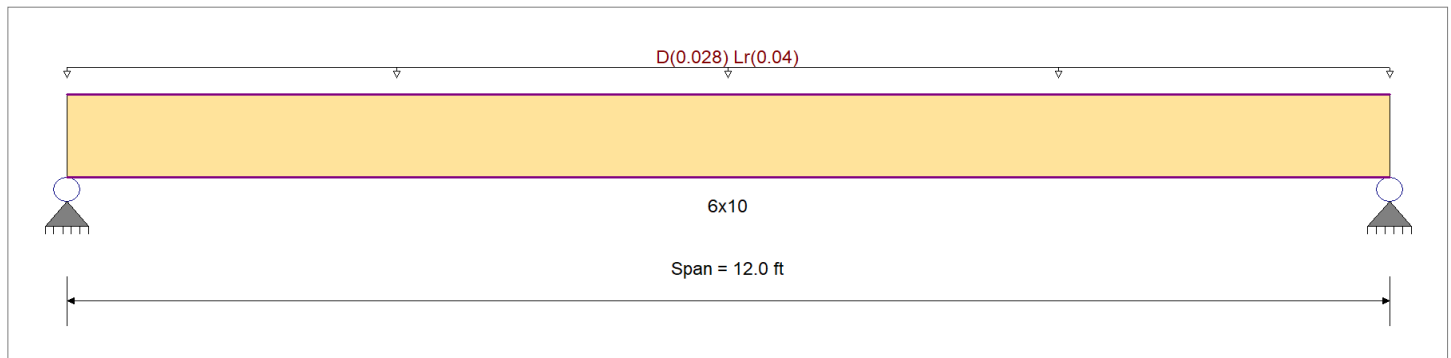
Material Properties

Analysis Method : Allowable Stress Design
Load Combination : ASCE 7-16

Wood Species : Douglas Fir-Larch
Wood Grade : No.1

Beam Bracing : Beam is Fully Braced against lateral-torsional buckling

Fb + 1,350.0 psi
Fb - 1,350.0 psi
Fc - Prll 925.0 psi
Fc - Perp 625.0 psi
Fv 170.0 psi
Ft 675.0 psi
E : Modulus of Elasticity
Ebend- xx 1,600.0ksi
Eminbend - xx 580.0ksi
Density 31.210pcf



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading

Uniform Load : D = 0.0140, Lr = 0.020 ksf, Tributary Width = 2.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio	=	0.123	1	Maximum Shear Stress Ratio	=	0.056	1
Section used for this span		6x10		Section used for this span		6x10	
fb: Actual	=	207.11 psi		fv: Actual	=	11.87 psi	
F'b	=	1,687.50 psi		F'v	=	212.50 psi	
Load Combination		+D+Lr		Load Combination		+D+Lr	
Location of maximum on span	=	6.000 ft		Location of maximum on span	=	0.000 ft	
Span # where maximum occurs	=	Span # 1		Span # where maximum occurs	=	Span # 1	
Maximum Deflection							
Max Downward Transient Deflection	0.030 in	Ratio =	4823	>=360	Span: 1 : Lr Only		
Max Upward Transient Deflection	0 in	Ratio =	0	<360	n/a		
Max Downward Total Deflection	0.059 in	Ratio =	2432	>=240	Span: 1 : +D+Lr		
Max Upward Total Deflection	0 in	Ratio =	0	<240	n/a		

Maximum Forces & Stresses for Load Combinations

Load Combination		Max Stress Ratios										Moment Values			Shear Values			
Segment Length	Span #	M	V	CD	CM	C _t	CLx	C _F	C _{fu}	C _i	C _r	M	fb	F'b	V	fv	F'v	
D Only															0.0	0.00	0.0	0.0
Length = 12.0 ft	1	0.085	0.038	0.90	1.00	1.00	1.00	1.000	1.00	1.00	1.00	0.71	102.7	1,215.0	0.20	5.9	153.0	
+D+Lr					1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0	
Length = 12.0 ft	1	0.123	0.056	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	1.43	207.1	1,687.5	0.41	11.9	212.5	
+D+0.750Lr					1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0	
Length = 12.0 ft	1	0.107	0.049	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	1.25	181.0	1,687.5	0.36	10.4	212.5	
+0.60D					1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0	
Length = 12.0 ft	1	0.029	0.013	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	0.42	61.6	2,160.0	0.12	3.5	272.0	

Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

(c) ENERCALC INC 1983-2022

DESCRIPTION: B1

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+D+Lr	1	0.0592	6.044		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1

Values in KIPS

Load Combination	Support 1	Support 2
Max Upward from all Load Conditions	0.476	0.476
Max Upward from Load Combinations	0.476	0.476
Max Upward from Load Cases	0.240	0.240
D Only	0.236	0.236
+D+Lr	0.476	0.476
+D+0.750Lr	0.416	0.416
+0.60D	0.142	0.142
Lr Only	0.240	0.240

Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

(c) ENERCALC INC 1983-2022

DESCRIPTION: B2

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Load Combination Set : ASCE 7-16

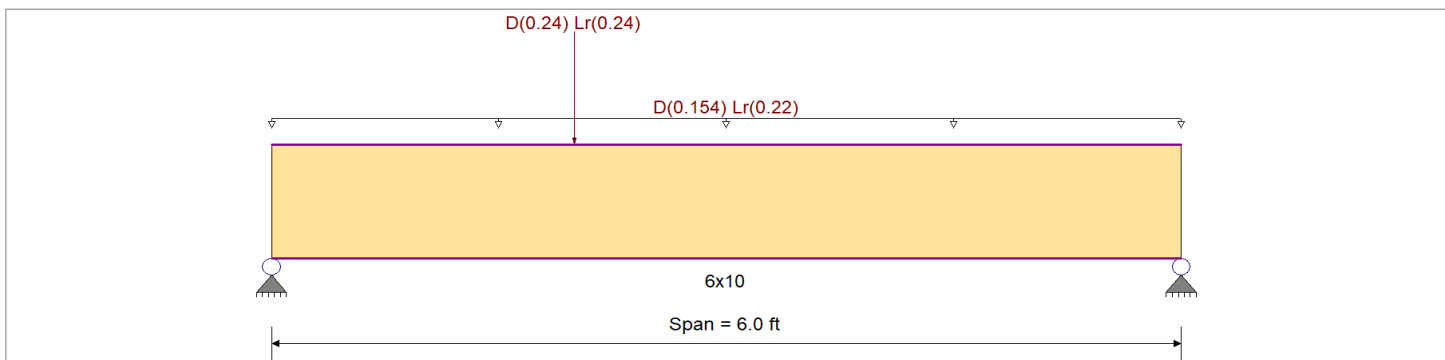
Material Properties

Analysis Method : Allowable Stress Design
Load Combination : ASCE 7-16

Wood Species : Douglas Fir-Larch
Wood Grade : No.1

Beam Bracing : Beam is Fully Braced against lateral-torsional buckling

Fb +	1,350.0 psi	E : Modulus of Elasticity	
Fb -	1,350.0 psi	Ebend- xx	1,600.0ksi
Fc - Prll	925.0 psi	Eminbend - xx	580.0ksi
Fc - Perp	625.0 psi		
Fv	170.0 psi		
Ft	675.0 psi	Density	31.210pcf



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading

Uniform Load : D = 0.0140, Lr = 0.020 ksf, Tributary Width = 11.0 ft

Point Load : D = 0.240, Lr = 0.240 k @ 2.0 ft, (PL FROM B1)

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio	=	0.193	1	Maximum Shear Stress Ratio	=	0.158	1
Section used for this span		6x10		Section used for this span		6x10	
fb: Actual	=	325.96psi		fv: Actual	=	33.65 psi	
F'b	=	1,687.50psi		F'v	=	212.50 psi	
Load Combination		+D+Lr		Load Combination		+D+Lr	
Location of maximum on span	=	2.584ft		Location of maximum on span	=	0.000 ft	
Span # where maximum occurs	=	Span # 1		Span # where maximum occurs	=	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.013 in	Ratio =	5619	>=360	Span: 1 : Lr Only
Max Upward Transient Deflection	0 in	Ratio =	0	<360	n/a
Max Downward Total Deflection	0.023 in	Ratio =	3120	>=240	Span: 1 : +D+Lr
Max Upward Total Deflection	0 in	Ratio =	0	<240	n/a

Maximum Forces & Stresses for Load Combinations

Load Combination		Max Stress Ratios										Moment Values			Shear Values		
Segment Length	Span #	M	V	CD	CM	C _t	CLx	C _F	C _{fu}	C _i	C _r	M	fb	F'b	V	fv	F'v
D Only														0.0	0.00	0.0	0.0
Length = 6.0 ft	1	0.120	0.099	0.90	1.00	1.00	1.00	1.000	1.00	1.00	1.00	1.00	145.5	1,215.0	0.53	15.1	153.0
+D+Lr					1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 6.0 ft	1	0.193	0.158	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	2.25	326.0	1,687.5	1.17	33.7	212.5
+D+0.750Lr					1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 6.0 ft	1	0.166	0.137	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	1.94	280.8	1,687.5	1.01	29.0	212.5
+0.60D					1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 6.0 ft	1	0.040	0.033	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	0.60	87.3	2,160.0	0.32	9.1	272.0

Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

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DESCRIPTION: B2

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+D+Lr	1	0.0231	2.956		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1

Values in KIPS

Load Combination	Support 1	Support 2
Max Upward from all Load Conditions	1.476	1.316
Max Upward from Load Combinations	1.476	1.316
Max Upward from Load Cases	0.820	0.740
D Only	0.656	0.576
+D+Lr	1.476	1.316
+D+0.750Lr	1.271	1.131
+0.60D	0.394	0.346
Lr Only	0.820	0.740

Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

(c) ENERCALC INC 1983-2022

DESCRIPTION: B3

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Load Combination Set : ASCE 7-16

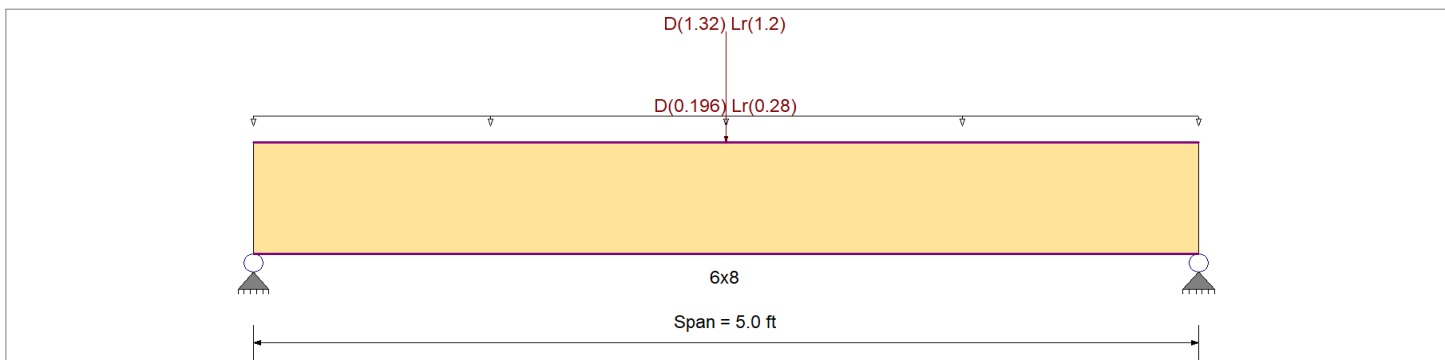
Material Properties

Analysis Method : Allowable Stress Design
Load Combination : ASCE 7-16

Wood Species : Douglas Fir-Larch
Wood Grade : No.1

Beam Bracing : Beam is Fully Braced against lateral-torsional buckling

Fb +	1,350.0 psi	E : Modulus of Elasticity	
Fb -	1,350.0 psi	Ebend- xx	1,600.0ksi
Fc - Prll	925.0 psi	Eminbend - xx	580.0ksi
Fc - Perp	625.0 psi		
Fv	170.0 psi		
Ft	675.0 psi	Density	31.210pcf



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading

Uniform Load : D = 0.0140, Lr = 0.020 ksf, Tributary Width = 14.0 ft

Point Load : D = 1.320, Lr = 1.20 k @ 2.50 ft, (PL From G.T.)

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio	=	0.643	1	Maximum Shear Stress Ratio	=	0.372	1
Section used for this span		6x8		Section used for this span		6x8	
fb: Actual	=	1,085.77 psi		fv: Actual	=	78.96 psi	
F'b	=	1,687.50 psi		F'v	=	212.50 psi	
Load Combination		+D+Lr		Load Combination		+D+Lr	
Location of maximum on span	=	2.500ft		Location of maximum on span	=	0.000 ft	
Span # where maximum occurs	=	Span # 1		Span # where maximum occurs	=	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.030 in	Ratio =	1976	>=360	Span: 1 : Lr Only
Max Upward Transient Deflection	0 in	Ratio =	0	<360	n/a
Max Downward Total Deflection	0.059 in	Ratio =	1016	>=240	Span: 1 : +D+Lr
Max Upward Total Deflection	0 in	Ratio =	0	<240	n/a

Maximum Forces & Stresses for Load Combinations

Load Combination		Max Stress Ratios										Moment Values			Shear Values			
Segment Length	Span #	M	V	CD	CM	C _t	CLx	C _F	C _{fu}	C _i	C _r	M	fb	F'b	V	fv	F'v	
D Only															0.0	0.00	0.0	0.0
Length = 5.0 ft	1	0.439	0.248	0.90	1.00	1.00	1.00	1.000	1.00	1.00	1.00	2.29	533.0	1,215.0	1.05	38.0	153.0	
+D+Lr															0.0	0.00	0.0	0.0
Length = 5.0 ft	1	0.643	0.372	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	4.67	1,085.8	1,687.5	2.17	79.0	212.5	
+D+0.750Lr															0.0	0.00	0.0	0.0
Length = 5.0 ft	1	0.562	0.323	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	4.07	947.6	1,687.5	1.89	68.7	212.5	
+0.60D															0.0	0.00	0.0	0.0
Length = 5.0 ft	1	0.148	0.084	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	1.37	319.8	2,160.0	0.63	22.8	272.0	

Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

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DESCRIPTION: B3

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+D+Lr	1	0.0590	2.518		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1

Values in KIPS

Load Combination	Support 1	Support 2
Max Upward from all Load Conditions	2.472	2.472
Max Upward from Load Combinations	2.472	2.472
Max Upward from Load Cases	1.300	1.300
D Only	1.172	1.172
+D+Lr	2.472	2.472
+D+0.750Lr	2.147	2.147
+0.60D	0.703	0.703
Lr Only	1.300	1.300

Wood Column

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

(c) ENERCALC INC 1983-2022

DESCRIPTION: 6x6 POST

Code References

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Analysis Method	Allowable Stress Design			Wood Section Name	6x6		
End Fixities	Top & Bottom Pinned			Wood Grading/Manuf.	Graded Lumber		
Overall Column Height	9 ft			Wood Member Type	Sawn		
(Used for non-slender calculations)							
Wood Species	Douglas Fir-Larch			Exact Width	5.50 in	Allow Stress Modification Factors	
Wood Grade	No.2			Exact Depth	5.50 in	Cf or Cv for Bending	1.0
Fb +	750.0 psi	Fv	170.0 psi	Area	30.250 in^2	Cf or Cv for Compression	1.0
Fb -	750.0 psi	Ft	475.0 psi	Ix	76.255 in^4	Cf or Cv for Tension	1.0
Fc - Prll	700.0 psi	Density	31.210 pcf	Iy	76.255 in^4	Cm : Wet Use Factor	1.0
Fc - Perp	625.0 psi			Incising Factors :		Ct : Temperature Fact	1.0
E : Modulus of Elasticity . . .		x-x Bending	y-y Bending	for Bending	0.80	Cfu : Flat Use Factor	1.0
	Basic	1,300.0	1,300.0	for Elastic Modulus	0.95	Kf : Built-up columns	1.0 NDS 15.3.2
	Minimum	470.0	470.0			Use Cr : Repetitive ?	No
Brace condition for deflection (buckling) along columns :							
				X-X (width) axis :	Unbraced Length for buckling ABOUT Y-Y Axis = 9 ft, K		
				Y-Y (depth) axis :	Unbraced Length for buckling ABOUT X-X Axis = 9 ft, K		

Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Column self weight included : 59.006 lbs * Dead Load Factor

AXIAL LOADS . . .

Axial Load at 9.0 ft, D = 0.660, Lr = 0.820 k

DESIGN SUMMARY

Bending & Shear Check Results

PASS	Max. Axial+Bending Stress Ratio =	0.09102 : 1	Maximum SERVICE Lateral Load Reactions . .			
	Load Combination	+D+Lr	Top along Y-Y	0.0 k	Bottom along Y-Y	0.0 k
	Governing NDS Forumla	Comp Only, fc/Fc'	Top along X-X	0.0 k	Bottom along X-X	0.0 k
	Location of max.above base	0.0 ft	Maximum SERVICE Load Lateral Deflections . . .			
	At maximum location values are .		Along Y-Y	0.0 in	at	0.0 ft above base
	Applied Axial	1.539 k	for load combination : n/a			
	Applied Mx	0.0 k-ft	Along X-X	0.0 in	at	0.0 ft above base
	Applied My	0.0 k-ft	for load combination : n/a			
	Fc : Allowable	558.95 psi	Other Factors used to calculate allowable stresses . . .			
			<u>Bending</u>	<u>Compression</u>	<u>Tension</u>	
PASS	Maximum Shear Stress Ratio =	0.0 : 1				
	Load Combination	+0.60D				
	Location of max.above base	9.0 ft				
	Applied Design Shear	0.0 psi				
	Allowable Shear	217.60 psi				

Load Combination Results

Load Combination	C _D	C _P	Maximum Axial + Bending Stress Ratios			Maximum Shear Ratios		
			Stress Ratio	Status	Location	Stress Ratio	Status	Location
D Only	0.900	0.866	0.05444	PASS	0.0 ft	0.0	PASS	9.0 ft
+D+Lr	1.250	0.799	0.09102	PASS	0.0 ft	0.0	PASS	9.0 ft
+D+0.750Lr	1.250	0.799	0.07890	PASS	0.0 ft	0.0	PASS	9.0 ft
+0.60D	1.600	0.728	0.02186	PASS	0.0 ft	0.0	PASS	9.0 ft

Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	X-X Axis Reaction		Y-Y Axis Reaction		Axial Reaction	My - End Moments		Mx - End Moments	
	@ Base	@ Top	@ Base	@ Top		@ Base	@ Top	@ Base	@ Top
D Only					0.719				
+D+Lr					1.539				
+D+0.750Lr					1.334				

Wood Column

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

(c) ENERCALC INC 1983-2022

DESCRIPTION: 6x6 POST

Maximum Reactions

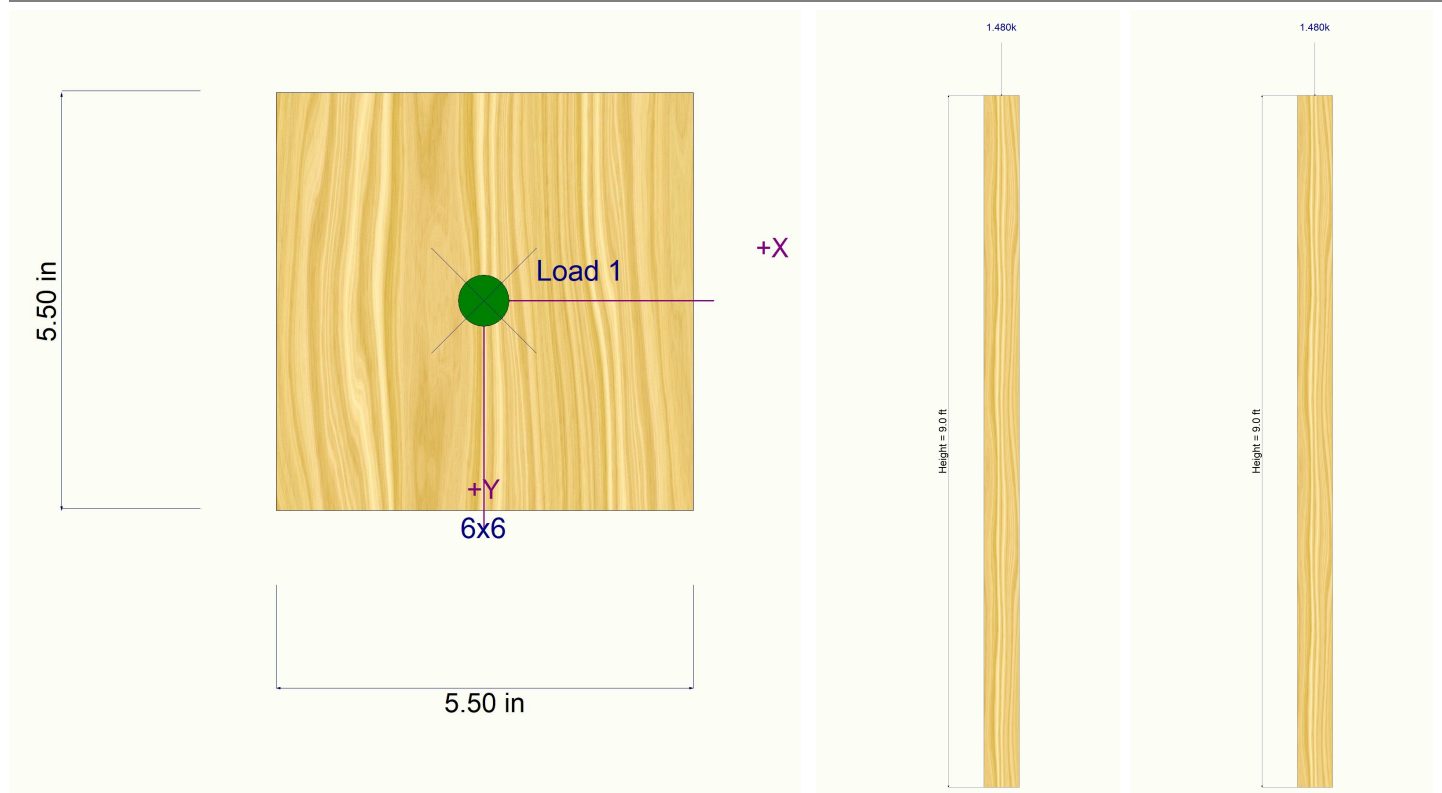
Note: Only non-zero reactions are listed.

Load Combination	X-X Axis Reaction		k	Y-Y Axis Reaction		Axial Reaction	My - End Moments		k-ft	Mx - End Moments	
	@ Base	@ Top		@ Base	@ Top	@ Base	@ Base	@ Top		@ Base	@ Top
+0.60D						0.431					
Lr Only						0.820					

Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection	Distance	Max. Y-Y Deflection	Distance
D Only	0.0000 in	0.000ft	0.000 in	0.000 ft
+D+Lr	0.0000 in	0.000ft	0.000 in	0.000 ft
+D+0.750Lr	0.0000 in	0.000ft	0.000 in	0.000 ft
+0.60D	0.0000 in	0.000ft	0.000 in	0.000 ft
Lr Only	0.0000 in	0.000ft	0.000 in	0.000 ft

Sketches





PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

SPREAD FOOTING FOUNDATION ANALYSIS:

Allowable Stress:

fy (ksi): 60
f'c (psi): 2500

Footing Reinf:

(2) #4 in Top
(2) #4 in Bottom

As used: 0.4

<i>Grid Line:</i>	<i>1</i>	<i>2</i>	<i>A</i>	<i>B</i>
<u>Footing Design Criteria:</u>				
Allow. Bearing Pres. (psf):	1500	1500	1500	1500
Footing Width (in):	12	12	12	12
Footing Depth (in):	18	18	18	18
Unsupported Length (ft):	5.0	5.0	5.0	5.0
<u>Uniform Loads:</u>				
Roof Trib (ft)	13.0	13.0	5.0	5.0
Roof DL (psf)	22.0	22.0	22.0	22.0
Roof LL (psf)	20.0	20.0	20.0	20.0
Floor Trib (ft)	0.0	0.0	0.0	0.0
Floor DL (psf)	15.0	15.0	15.0	15.0
Floor LL (psf)	40.0	40.0	40.0	40.0
Lower Floor Trib (ft)	0.0	0.0	0.0	0.0
Lower Floor DL (psf)	15.0	15.0	15.0	15.0
Lower Floor LL (psf)	40.0	40.0	40.0	40.0
Deck Trib (ft)	0.0	0.0	0.0	0.0
Deck DL (psf)	15.0	15.0	15.0	15.0
Deck LL (psf)	60.0	60.0	60.0	60.0
Wall Trib (ft)	10.0	10.0	10.0	10.0
Wall DL (psf)	17.0	17.0	17.0	17.0
TOTAL DL (plf):	456	456	280	280
TOTAL Lr (plf):	260	260	100	100
TOTAL LL (plf):	0	0	0	0
<u>Point Load:</u>				
Max. Point DL (lbs)	1320	1320	0	0
Max. Point Lr (lbs)	1200	1200	0	0
Max. Point LL (lbs)	0	0	0	0
Max. HD Load (lbs)	2153	3415	4863	5434
<u>Soil Analysis:</u>				
q (psf):	1331	1331	851	951
q/ q _{ALLOW} :	0.89	0.89	0.57	0.63
M_{max} (ft-lb):	7391 ft-lb	7391 ft-lb	5360 ft-lb	5897 ft-lb
V_{max} (lb):	4161 lbs	4161 lbs	2460 lbs	2675 lbs
Allowable M_{max} (ft-lb):	25253 ft-lb	25253 ft-lb	25253 ft-lb	25253 ft-lb
Allowable V_{max} (lb):	7395 lbs	7395 lbs	7395 lbs	7395 lbs
Footing A(s) req'd:	0.40	0.40	0.40	0.40
	O.K.	O.K.	O.K.	O.K.



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

PIER AND GRADE BEAM FOUNDATION ANALYSIS:

Allowable Stress:

fy (ksi): 60
f'c (psi): 2500

Grade Beam Reinf:

(3) #5 in Top
(3) #5 in Bottom

As used: 0.93

Grid Line:	1	2	A	B
Pier Design Criteria:				
Allow. Skin Friction (psf):	250	250	250	250
Pier Diameter (in):	16	16	16	16
Neglect Pier Depth (ft):	2	2	2	2
Grade Beam Design Criteria:				
Grade Beam Width (in):	12	12	12	12
Grade Beam Depth (in):	18	18	18	18
Grade Beam Span (ft):	8.0	8.0	8.0	8.0
Uniform Loads:				
Roof Trib (ft)	13.0	13.0	5.0	5.0
Roof DL (psf)	22.0	22.0	22.0	22.0
Roof LL (psf)	20.0	20.0	20.0	20.0
Floor Trib (ft)	0.0	0.0	0.0	0.0
Floor DL (psf)	15.0	15.0	15.0	15.0
Floor LL (psf)	40.0	40.0	40.0	40.0
Lower Floor Trib (ft)	0.0	0.0	0.0	0.0
Lower Floor DL (psf)	15.0	15.0	15.0	15.0
Lower Floor LL (psf)	40.0	40.0	40.0	40.0
Deck Trib (ft)	0.0	0.0	0.0	0.0
Deck DL (psf)	15.0	15.0	15.0	15.0
Deck LL (psf)	60.0	60.0	60.0	60.0
Wall Trib (ft)	10.0	10.0	10.0	10.0
Wall DL (psf)	17.0	17.0	17.0	17.0
TOTAL DL (plf):	681	681	505	505
TOTAL Lr (plf):	260	260	100	100
TOTAL LL (plf):	0	0	0	0
Point Load:				
Max. Point DL (lbs)	1320	1320	0	0
Max. Point Lr (lbs)	1200	1200	0	0
Max. Point LL (lbs)	0	0	0	0
Max. HD Load (lbs)	2153	3415	4863	5434
Grade Beam Analysis:				
M_{max} (ft-lb):	16877 ft-lb	16877 ft-lb	10959 ft-lb	11817 ft-lb
V_{max} (lb):	6686 lbs	6686 lbs	3651 lbs	3866 lbs
Allowable M_{max} (ft-lb):	56104 ft-lb	56104 ft-lb	56104 ft-lb	56104 ft-lb
Allowable V_{max} (lb):	7395 lbs	7395 lbs	7395 lbs	7395 lbs
Grade Beam A(s) req'd:	0.93	0.93	0.93	0.93
	O.K.	O.K.	O.K.	O.K.
Pier Analysis:				
Design Pier Load (lbs):	10051	10051	5598	5898
Req'd Pier Depth (ft)	11.60	11.60	7.35	7.64
Use Pier Depth (ft):	12	12	8	8